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editor's note



What's your story? Well for starters, if you're reading this, I'm guessing you might just be a structural engineer—and if not, you're almost certainly involved in steel design or construction in some capacity. Or perhaps you were just looking to jump out of your comfort zone? (If so, thanks! Tell your friends and neighbors about us.)

Full disclosure: I'm not a structural engineer. Or a steel fabricator. Or an erector, or a detailer, or an architect.

But I have been *Modern Steel's* senior editor for more than a decade, and I appreciate buildings and bridges. Occasionally, *Modern Steel* Editor Scott Melnick turns this space over to a new voice. And since I like writing and telling stories, it's my turn.

I also like people. In my time with AISC and *Modern Steel*, we've featured countless stories related to steel buildings and bridges—stunning designs, means and methods for making steel projects easier and faster to create, in-depth looks at specific steel framing components, and behind-the-scenes tours of various processes and facilities along the steel supply chain. We've recently made an effort to tell the stories of the people who make steel projects better, many of whom also make the industry, and even the world, better.

And for the past year, we've been doing just that via our Field Notes podcasts. We told the story of how a longtime fabricator became the longtime head judge of AISC's Student Steel Bridge Competition (for more on the competition, visit aisc.org/ssbc). And the story of how two engineering students who met sitting next to each other in a class went on to found (forge?) a word-class castings company. And how the CEO of an HSS giant is working to bring manufacturing jobs back to the U.S. And how a psychiatric nurse

became the head of a major Northeastern fabrication company. And how two visionary architects developed an inspiring update to a New York icon, one that was impressive enough to win AISC's Forge Prize Award (www.forgeprize.com). And several others. (You can find all of the podcasts at www.modernsteel.com.)

In this month's issue, we're telling the story of how veteran ironworker Vicki O'Leary is changing the face of the profession and making it a safer, more equitable landscape for all.

Plenty of great stories will also be told by the best minds in the steel industry at the 2021 NASCC: The Virtual Steel Conference, taking place April 12–16. Visit aisc.org/nascc to register and take a gander at the more than 150 sessions on offer. (Oh, and you can also see a handful of preview articles—including our SteelWise and Business Issues columns—and the exhibitor list in this issue.)

So again, what's your story? A marquee project held up by thousands of tons of steel? A connection you designed that's so genius it borders on crazy? A eureka methodology you developed that can be implemented on multiple projects? Email me (weisenberger@aisc.org) and let me know. Let us share it.


Geoff Weisenberger
Senior Editor

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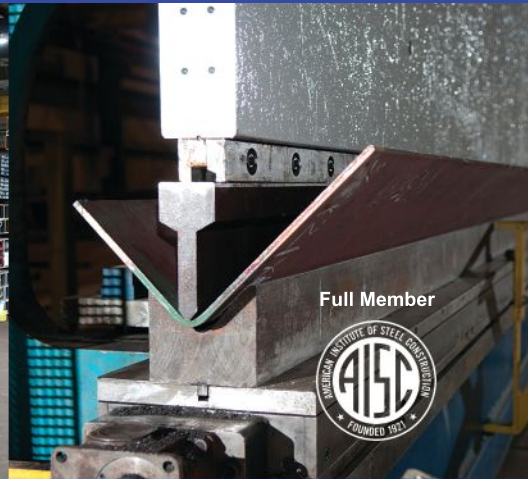
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If you've ever asked yourself "Why?" about something related to structural steel design or construction, *Modern Steel's* monthly Steel Interchange is for you!

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steel interchange

All mentioned AISC codes and standards, unless noted otherwise, refer to the current version and are available at aisc.org/specifications. All mentioned Engineering Journal articles are available at aisc.org/ej, and AISC Design Guides are available at aisc.org/dg.

Restraint Classifications

Figure C-B3.3 in Commentary Section B3.4 of the AISC Specification for Structural Steel Buildings (ANSI/AISC 360) shows the criteria for a simple shear connection, "PR" moment connection, and "FR" moment connection. But the specifications do not state the source of "2EI/L" or "20EI/L." Can you provide some references regarding this?

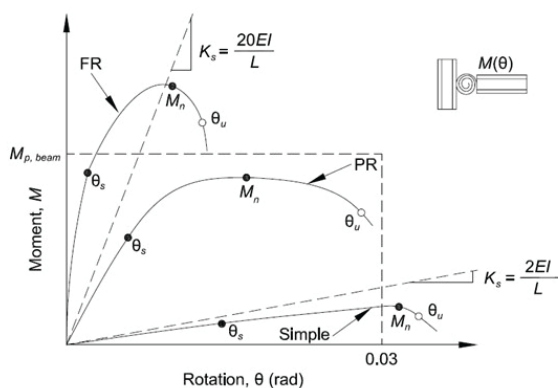


Fig. C-B3.3. Classification of moment-rotation response of fully restrained (FR), partially restrained (PR), and simple connections.

Yes. Several different connection classification systems are available. Commentary Section B3.4 of the AISC Specification refers to a paper by Leon (1994). One of the most used systems is summarized in the book by Chen et al. (2011). The basis of the values in Commentary Figure C-B3.3 is the paper by Bjorhovde et al. (1990). The other two primary classification systems were developed by Goto et al. (1998) and Nethercot et al. (1998).

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- Bjorhovde, R., Colson, A. and Brozzetti, J. (1990), "Classification System for Beam-to-Column Connections," *Journal of Structural Engineering*, Vol. 116, No. 11, ASCE.
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Bo Dowsell, PE, PhD

Global Warming Potential Difference in an HSS Environmental Product Declaration (EPD)

Why is the global warming potential for hollow structural sections (HSS) different in the AISC environmental product declaration (EPD) than in the Steel Tube Institute's (STI) EPD if the AISC EPD was developed in conjunction with STI?

The EPDs from both organizations were indeed developed together. AISC's adds the impacts from transport to fabricator and final fabrication, whereas the Steel Tube Institute's EPD scope ends at the HSS manufacturer. You can view AISC's HSS EPD at aisc.org/why-steel/resources/leed-v4, and STI's at steeltubeinstitute.org/resources/leed-epd.

Max Puchtel, SE

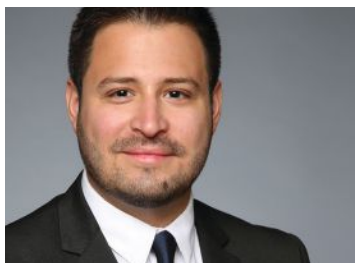
Effective Weld Area of a Flare Bevel Groove Weld

I'm designing a welded connection between the corners of an HSS member to a steel plate and was wondering how I would determine the effective weld area used in determining the available strength per the AISC Specification. I know the welding process used, but I am not sure how to calculate the weld metal's effective throat.

Effective weld area will be equal to the effective throat times the length of the weld. Section 17.3.7 in AISC Design Guide 21: *Welded Connections—A Primer for Engineers* states: "Flare V- and flare-bevel groove welds, when filled flush, have throat dimensions that are defined in AISC Specification Table J2.2. The throat dimension depends on the welding process and is a function of the radius on the curved member(s). It is often assumed that filling these joints flush is required, but less than flush-filled joints are permitted, provided that the required throat is still obtained. This cost-saving idea is simple—specify only the required throat dimension rather than assuming that the flush condition is required. As the thickness of the curved member increases, so do the radius and weld throat that is achieved for heavier members. As a result, the potential savings can be significant when thicker members are involved (see Section 3.4.5 of this Guide)."

Table 8-2 in the AISC Manual provides a detail of a prequalified welded flare-bevel groove weld joint on page 8-62. The effective throat, (E), provided in the table is based on the weld being filled flush and is consistent with the values determined in Specification Table J2.2. While only a flush-filled condition is illustrated in Table 8-2, the Specification does permit welds less than flush-filled as described in AISC Design Guide 21.

Jonathan Tavares, PE



Max Puchtel (puchtel@aisc.org) is AISC's director of government relations and sustainability, and **Jonathan Tavarez** (tavarez@aisc.org) is a staff engineer with AISC's Steel Solutions Center. **Bo Dowswell**, principal with ARC International, LLC, and **Larry Muir** are consultants to AISC.



STEEL SOLUTIONS CENTER

Steel Interchange is a forum to exchange useful and practical professional ideas and information on all phases of steel building and bridge construction. Contact Steel Interchange with questions or responses via AISC's Steel Solutions Center: 866.ASK.AISC | solutions@aisc.org

The opinions expressed in Steel Interchange do not necessarily represent an official position of the American Institute of Steel Construction and have not been reviewed. It is recognized that the design of structures is within the scope and expertise of a competent licensed structural engineer, architect or other licensed professional for the application of principles to a particular structure.

The complete collection of Steel Interchange questions and answers is available online at www.modernsteel.com.

SCBF Beam-to-Column Connection

When considering special concentrically braced frames (SCBFs) for our beam-to-column connection, we see that Section F2.6b(a) pg. 9.1-67 of the AISC *Seismic Provisions for Structural Steel Buildings* (ANSI/AISC 341) states that the connection assembly shall be a simple connection meeting the requirements of *Specification* Section B3.4a, where required rotation is taken to be 0.025 rad. Would a bolted-welded single plate connection meet this rotation requirement?

Yes, a single-plate connection designed following the guidance provided in Part 10 of the 15th Edition AISC *Steel Construction Manual* can satisfy this requirement, although the gusset-to-column connection should be considered as well. Significant effort was dedicated to writing Commentary to the *Seismic Provisions* and providing similar language in various portions of the AISC documents to indicate a relationship between these sections relative to this issue.

For example, the Commentary to *Provisions* Section D3 states: "Properly designed simple connections are required at beam-to-column joints to avoid significant flexural forces. As per *Specification* Section J1, inelastic deformation of the connections is an acceptable means of achieving the required rotation. Standard shear connections, per Part 10 of the AISC *Steel Construction Manual*, can be considered to allow adequate rotation at the joints without significant flexural moments. Double angles supporting gravity loads have been shown to attain maximum rotations of 0.05 to 0.09 rad and are suitable for combined gravity and axial forces, as are WT connections, which have demonstrated rotations of 0.05 to 0.07 rad (Astaneh-Asl, 2005a). Shear-plate connections (single plates), while inherently more rigid than double angles, have been shown to withstand gravity rotations ranging from 0.026 to 0.103 rad and cyclic rotations of 0.09 rad (Astaneh-Asl, 2005b). Note that reducing the number of bolts in shear plates, and consequently the connection depth, increases the maximum possible rotation. Other connections at beam-to-column joints are acceptable if they are configured to provide adequate rotational ductility. Part 9 of the AISC *Manual* provides guidance on rotational ductility of end plate and WT connections that can be applied to many types of connections to ensure ductile behavior."

The Commentary to the *Seismic Provisions* Section F2.6b states: "The provision allows the engineer to select from three options. The first is a simple connection (for which the required rotation is defined as 0.025 rad). The connections presented in the *Manual*, Part 10, are capable of accommodating rotations of 0.03 rad and therefore meet the requirement for a simple connection. However, it is important to recognize that in many configurations, the gusset and beam behave rigidly relative to one another such that the beam-to-column connection and the gusset-to-column connection should be treated similarly with respect to deformation demands to achieve rotational ductility."

Part 9 of the *Manual* states: "Connections satisfying the parameters discussed in the foregoing can be expected to accommodate rotations in the range of 0.03 rad. The checks are intended for use with connections between 6 in. and 36 in. deep and configured similarly to the connections shown in Part 10. The use of deeper connections, smaller offset distances between the supported and supporting members, or smaller edge distances can affect the ability of connections to accommodate large rotations in a ductile manner. Connections satisfying these parameters satisfy the intent of AISC *Specification* Section B3.4a for simple connections."

Larry Muir, PE

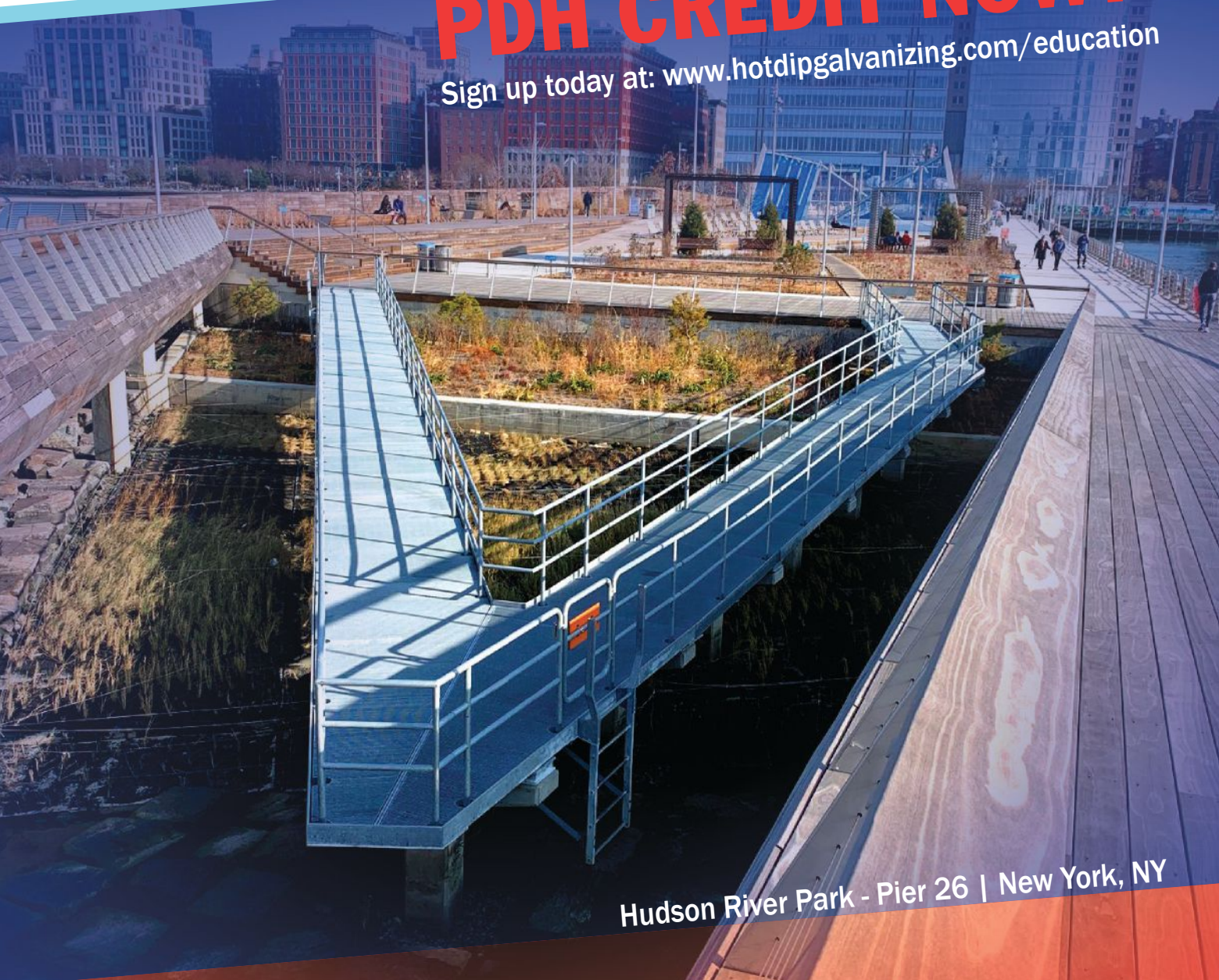
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steel quiz

This month's quiz focuses on the new version of the *AISC Standard for Certification Programs* (AISC 207-20), which was published last year. You can download the standard at aisc.org/publications.

- 1 The 2020 edition of the AISC 207 standard contains a new chapter addressing which of the following? (Choose one.)
- a. Metal component manufacturer
 - b. Fracture-critical requirements
 - c. Hydraulic metal structures
 - d. Sophisticated painting

2 **True or False:** The quality management system may only address *either* quality control or quality assurance so long as the scope is clearly defined.

3 **True or False:** Commonly used shop abbreviations should be included in the detailing standards.

4 **True or False:** AISC, through the *Certification Standard*, sets a required length of time for document retention.

5 **True or False:** Metal component manufacturers who manufacture standard components that are not specific to any particular project are not required to have the design sealed by a registered design professional, so long as the design tables or processes are readily available and included in the component literature.


6 **True or False:** A bridge comprised of haunched built-up plate girders with a radius of 1,200 ft requires a higher level of fabrication expertise and should be classified as an advanced bridge.

7 **True or False:** For fabricators of fracture-critical members, the welding procedure specifications must contain a procedure qualification record for all fracture-critical welds.

8 **True or False:** A fabricator must have at least one Level III and one Level II Certified NDT on staff or available under contract for each nondestructive testing (NDT) method performed in the shop.


9 **True or False:** The fracture-control plan included in a hydraulic metal component fabricator's documented welding procedure shall be in accordance with the AWS D1.5 *Bridge Welding Code*.

TURN TO PAGE 14 FOR THE ANSWERS



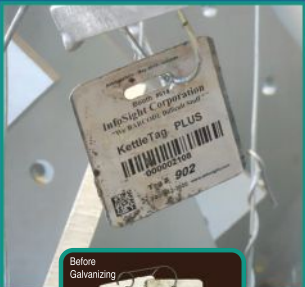
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steel quiz ANSWERS

- c.** Hydraulic metal structures.
- False.** Section 1.5.2 states that the quality management system must address both quality control and quality assurance as defined in the glossary.
- True.** Section 1.7.1 includes commonly used shop abbreviations in the list of items that shall be described in the fabricator's or manufacturer's methods of drawing layout.
- False.** Section 1.9.1 simply states that a documented procedure for the maintenance of quality records shall define the retention policy and provisions for the disposition of the records at the end of the retention period.
- False.** The design of both standard and nonstandard components must be reviewed and sealed by a registered design professional. These requirements are discussed further in Sections 3.7.9 and 3.7.10.
- False.** The fabricator would be classified as an "Intermediate Bridge Fabricator." The Commentary to Section 4.2 provides examples of intermediate bridges, the parameters of which are satisfied by the bridge described in the question statement.
- True.** This is required per Section 4F.12.1(a).
- True.** This is required in Section 6.5.4.1.
- True.** This is required in Section 6.F.12.1, except that all instances of the word "bridge" shall be replaced with "hydraulic structure," and the first sentence of the Certification and Qualification section shall be omitted.

Everyone is welcome to submit questions and answers for the Steel Quiz. If you are interested in submitting one question or an entire quiz, contact AISC's Steel Solutions Center at 866.ASK.AISC or solutions@aisc.org.

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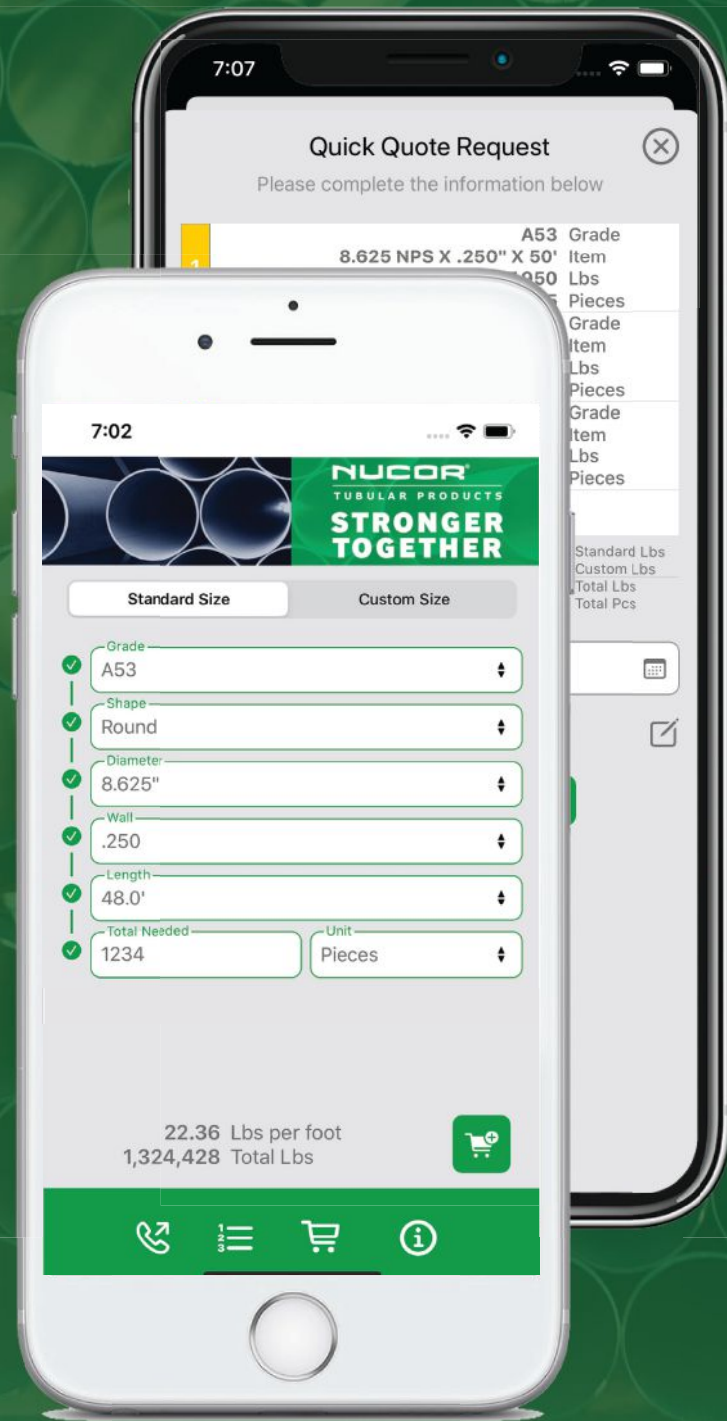
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BY JONATHAN STRATTON,
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A new series of AASHTO/NSBA documents will provide guidance on common repairs to extend steel bridge service life.

THE 2018 AASHTO/NSBA Steel Bridge Collaboration meeting in Minneapolis involved a spirited discussion amongst the members of Task Group (TG) 12: Design for Constructability and Fabrication—and resulted in a series of G14 documents focused on field repairs and retrofits of steel bridges.

At the discussion's conclusion, a challenge was issued to the group: Define a direction for a potential G12.2 document. (The Guideline “G” documents contain best practices for the design, fabrication, and construction processes, using a common language across all stakeholders, including engineers, owners, fabricators, and erectors. To view all of the Guideline “G” documents, visit aisc.org/gdocs.)

The membership passed on large-scale options such as cable-stayed bridges, truss and arched structures, and even the all-encompassing topic of complex bridges. One suggestion from the floor was tubular structures—but that failed to gain traction as well.

The discussion digressed, eventually leading to the sharing of an observation: Just over a decade before the meeting, and only a short distance from its location, one of the most significant bridge collapses in U.S. history had occurred. The I-35W bridge over the Mississippi River collapsed, succumbing to excessive loading while undergoing deck, lighting, and guard rail repairs. Acknowledging the 2007 tragedy, in which 13 were killed and 145 were injured, provided the direction for the guidance document that the group would create: steel bridge repair and retrofit. (The task group that would eventually take on this endeavor is TG 14: Field Repairs and Retrofits of Steel Bridges.)

NSBA had actually considered such an endeavor a decade earlier. The original TG14 was successful in generating a legacy with their development and distribution of a survey with responses from various state DOTs. It was these responses that supported the newly activated TG14 membership to define the content for the upcoming series of documents.

The current TG14's membership consists of a spectrum of industry stakeholders and has an agenda to produce documents that speak to the on-site project team, the steel fabricator, the designer, and all the way up to the bridge owner and every project participant in between. The effort moving forward is to present three distinct but complementary documents.

The initial document will focus on fatigue and constraint-induced fracture (CIF). NSBA recently identified the document *Maintenance Actions to Address Fatigue Cracking in Steel Bridge Structures* (prepared under National Cooperative Highway Research Program (NCHRP) Project 20-07, Task 387) as a candidate for a Collaboration document, G14.1. This document provides relevant insight into the causes, identification, and remediation of deficiencies resulting from fatigue, as well as the potential risk of CIF. (Note that at the time of this article's publication, the NCHRP document is going through the AASHTO balloting process for acceptance as an AASHTO/NSBA Collaboration document and is anticipated to be accepted with some relatively minor comments.) The proposed G14.1 is a synthesis of best practices from published literature, project reports, and past and ongoing research projects, as well as input from industry professionals. Several case studies are discussed, providing context for the different detail susceptibilities and using a mixture of real-world and rendered images to illustrate the problems and solutions. For each case, a suggested sequence of steps is also provided as a “how to.”

G14.1

Here’s an abridged example of one maintenance action in the proposed G14.1 for fatigue cracking at a cover plate termination detail.

A slip-critical bolted splice is a reliable retrofit that can be installed at a cover plate detail. As seen in Figure 1, an existing fatigue crack has propagated into the web plate. A web and flange splice is recommended, sufficiently designed to carry the full moment of the girder at that location. It would also be highly recommended to drill a hole at the crack tip(s) to arrest the crack, as research has shown that a bolted splice installed after fatigue cracking has initiated may not prevent subsequent crack growth. Adding a 2-in- to 4-in.-diameter drilled hole in the web directly above (or below in cases where the cover plate is on a top flange) the cover plate weld toe could also be done to ensure that any future crack growth cannot extend into the web plate. Figure 1 shows a fatigue crack that has grown through the thickness of the flange plate and is arrested in the drilled hole. Figure 2 renders what a typical fully bolted flange splice over the cover plate termination might look like.

G14.2

The second document, G14.2, will add to a broad spectrum of repairs and retrofits—including damage from vehicle impact, overloading, and section loss—while also providing guidance for unique component-specific repairs such as bearings and truss members. Damage can vary widely in type and scope, and defining an efficient plan to remediate the damage can be challenging. G14.2 will offer guidance to engineers for repair alternatives that have been successfully implemented. For example, Figure 3 shows the aftermath of an over-height vehicle that struck a through-truss, damaging several members. The portal strut reflects initial contact, and severe deformation of the second and third struts was caused by the front end of the garbage truck being lifted while the rear end was snagged by the first strut. In some cases, member replacement is the only option (as was the case for this bridge). However, heat straightening or mechanical straightening techniques can sometimes be implemented to quickly and safely restore a member to its original geometry without compromising strength and durability. G14.2 will provide the guidance to help owners and designers when faced with a similar scenario.

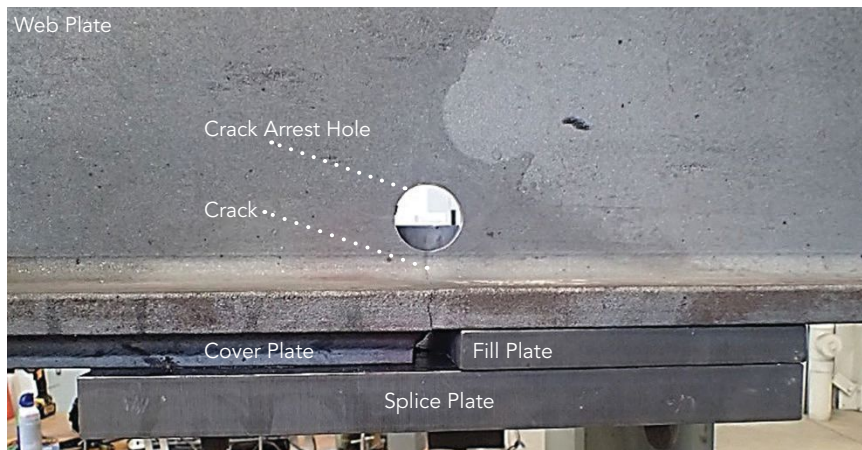


Fig. 1. A crack-arrest hole preventing a crack from propagating into the web plate.

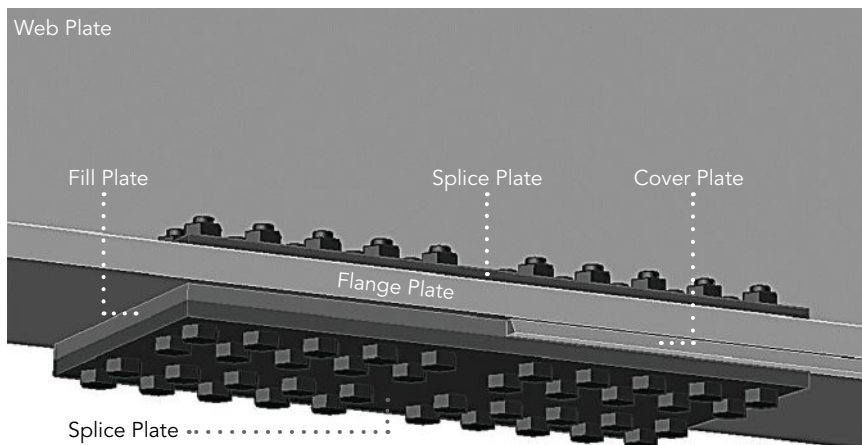


Fig. 2. A detail of a full-flange splice at a cover plate.



Fig. 3. Impact damage to through-truss struts by an over-height vehicle.

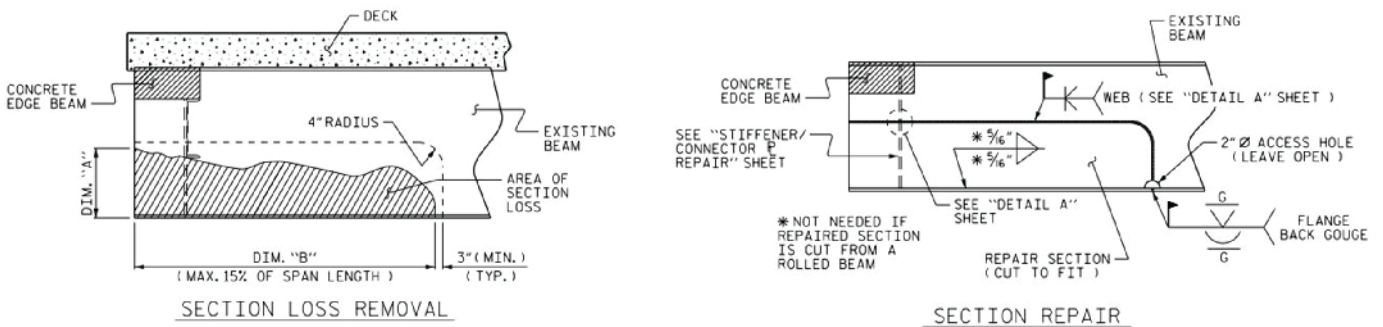


Fig. 4. A typical beam-end corrosion-damage repair detail.

Another relatively common repair performed by owners is girder-end repair. Section loss at girder ends is a challenge faced by every bridge owner and is often caused by leaking joints that allow deicing salts to flow over the steel girder ends. Effective bridge maintenance programs can help mitigate corrosion and can also be key to reducing damage by providing early remediation of compromised coatings. Figure 4 is an example of a common detail used for

girder end repair that will be discussed in the proposed G14.2 document.

There are several considerations for making this type of repair. G14.2 will offer guidance for engineers that will help identify important considerations for each repair. For example, flexibility in material selection could be an important consideration for a beam-end repair, as both the fabricator and erector are directly impacted when options are limited—and

it can be challenging to find matching historic steel shapes in older bridges. If a specific rolled section is required, then the designer must ensure that it is actively being rolled and is commercially available. If the existing member is a plate girder and time is of the essence for project success, then providing a range of acceptable plate thicknesses will support timely production. Other considerations include defining the extent of shop and field coating, where and by which party the weld edge preparation is to be performed, and assuring accessibility for all field tasks, including nondestructive evaluation (NDE).

G14.3

The third target document, G14.3, will be more graphical in nature than G14.1 and G14.2, effectively becoming an evolving catalog of repair and retrofit details that have been used by engineers throughout the country. The intent is to complement the text and details of G14.2 with supplemental drawings and details, some procedural insight, and commentary on a relative-cost basis, effectiveness, ease of performance, and considerations to be made when selecting the defined repair.

Steel bridge superstructures lend themselves well to repair and retrofit methods. The repairs mentioned are just a small sample of what the G14 documents are built around, a compilation of best practices augmented with commentary on effectiveness and design considerations. The result will be a powerful resource for the various stakeholders charged with extending the service lives of the country’s steel bridges.

This article is a preview of the 2021 NASCC: The Steel Conference session “Common Steel Bridge Repairs to Extend Service Life.” The conference takes place online April 12-16. For more information and to register, visit aisc.org/nascc.

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field notes

RAISING THE BAR

INTERVIEW BY
GEOFF WEISENBERGER

Vicki O’Leary’s mission is to make the ironworking profession more equitable and safer, one heart and mind at a time.



Field Notes is *Modern Steel Construction’s* **podcast series**, where we interview people from all

corners of the structural steel industry with interesting stories to tell. Listen in at modernsteel.com/podcasts.



Geoff Weisenberger
(weisenberger@aisc.org) is senior editor of *Modern Steel Construction*.

VICKI O’LEARY, a member of Iron Workers Local #1 in Chicago, was born into an ironworking family and got into the profession herself based on a bet with her brother. And while she’s raised a lot of steel in her more than thirty years as an ironworker, more recently she has raised the bar for ironworkers everywhere as director of diversity for Ironworkers International. In this month’s Field Notes podcast, she discusses how she got into the profession, the Be That One Guy anti-bullying program, and the evolution of paid maternity for ironworkers.

I understand you’ve been in Chicago for a while. Are you from here?

I was actually born in Paradise, Calif. When I was six weeks old, we moved to Chicago, and from there we ended up moving to Kenosha, Wis., and then back to Chicago. And my parents are from Arkansas, so we moved there for a while—and then, again, back in Chicago.

Sounds like you keep getting drawn back here! I understand you had a triple major in college and also have a master’s degree. What did you study?

I knew that I wanted to move forward in my career, and I figured that I had to bring as much to the table as possible to stand out. So I went to National Labor College [in Maryland] and got my degrees in union leadership and administration, labor studies, and labor education. Then I went on to Gonzaga University and got my master’s degree in organizational leadership. And then I also graduated from the Harvard Trade Union Program as well as Cornell’s National Labor Leadership initiative. I think I’ve run the gamut of education!

Let's go back a bit further. How did you get into the ironworking trade?

I got into the ironworkers on a bet between me and my brother. The four of us were at the dinner table—my mom, my dad, my brother, and myself—and my brother, an ironworker, bet me that I couldn't do it. So basically, it was game on. My brother and I have always been very competitive, and he's three years older than me. We were raised to believe that occupations don't have gender limitations, so with that, I was going to prove him wrong. And it ended up being my career. I even did better than my brother on the apprenticeship test. I started working as an ironworker when I was 20 and didn't go back to school until I was in my 40s.

Can you tell me a little bit about your first experience up on the iron?

There's usually a long gap between taking your tests to get into a trade and actually starting the trade. After taking my apprenticeship test, I was working at a law firm as a legal secretary and I got a call at work, and they said they wanted me to go to work the next day at McCormick Place in Chicago; this was the expansion in 1985. And during that first experience being up on the iron, I was exhilarated, I was nervous, I was anxious. I was extremely proud of myself, and I remember watching the connectors and it was like watching a well-orchestrated ballet. They seemed to be so in sync with each other. It was also pretty cool that I was in that first group of women that started in Local #1. I'm thankful that on that job, I saw examples of what workers should be like on a site—unfortunately, it's not always like that—but they were actually pretty cool to us as the first group of women coming through.

Another early job was on a 40-story building in the heart of the financial district in Chicago. This is where I learned firsthand about the perception of people who work in construction from people who wear suits—I used to be that person wearing a suit in a law firm on Michigan Avenue—as a construction worker trying to go to a restaurant for lunch or trying to find a restroom. It was funny because I was making more money than I ever had in my life. I had more money in my pocket than when I worked at the law firm, but the majority of the suit people didn't give construction workers any kind of respect.

As an ironworker, do you have to go into the job without a fear of heights, or is that something you can gradually get over?

On that 40-story job, I remember when we got to the 28th floor and we were going to jump to the 29th floor, and I wasn't scared until I got to the 29th floor. It took a few days to get over why that extra floor made a difference for me. But after that, I was okay. And that was in 1985 before fall protection became a thing, so we weren't wearing harnesses.

Switching gears a bit, can you talk about the Be That One Guy program and what sparked it?

The day that Outi Hicks was killed by one of her coworkers is one that no woman working in the construction trades will ever forget. Outi was a carpenter apprentice in Fresno, and she was bludgeoned to death by a guy she was working with on a job on Valentine's Day of 2017. I've asked women ironworkers if they

were ever afraid on the job site outside of the inherent dangers of the work they do, and all of them said yes. There are a lot of great women ironworkers that have left the industry, not because they weren't good at it or because the work itself was too difficult but because of the way they were treated. They were fine with working hard up high and in small spaces, but they weren't okay with the disrespect and abuse from others. So we developed the Be That One Guy program because we recognized that harassment, intimidation, and other kinds of bullying on the job can make for a hostile work environment. And we recognize that it's not just a treatment issue but also a safety hazard. Our approach to developing the program included the same methods as our safety program training and empowering everyone to address on-the-job harassment. Not everyone addresses it in the same manner, and we're trying to teach people that that's okay. Even if it's just deflecting from a situation to get the person that's being harassed or bullied out of that situation, that's fine because not everybody is an in-your-face kind of person to say, "Knock it off."

Can you talk a bit about how you've influenced Ironworkers International's maternity policy?

There's a tragic story about a woman ironworker losing her unborn child while working because as a single mom, she had no choice but to go to work if she wanted to provide for her family. So from that story, the question was posed to our general president, Eric Dean, that if we can implement an on-the-job injury program to assist ironworkers, why not also have a maternity program that provides similar assistance to our female members. The end result was our paid maternity leave program. It's funded through currently established contributions with no additional cost for either the contractor or the member, and the program brings relief to our pregnant members by providing financial support that can help families redeem their health insurance. It ensures the safety of both the mother and her baby during pregnancy and allows members to fully recover from delivery prior to returning to work. The program relieves contractors from possible liability by allowing ironworkers to be open about their pregnancies without fear of losing their health-care coverage. Prior to the establishment of the program, members couldn't be honest with their employer about their pregnancy, out of concern for their employment status and continued hours, and then they weren't being honest with their doctors about how grueling and how hard their work is. So we help our partner contractors retain training dollars by creating a safe and healthy alternative for ironworker employees during their pregnancy and encouraging return to work following delivery. IMPACT (the Ironworker Management Progressive Action Cooperative Trust) handles the program claim forms from start to finish, which takes the administrative burden off the contractors and local unions. It's a win-win. This isn't an insurance policy. This is a safety program, and like any other safety program, it's designed to help both contractors and workers. It's just good business sense. ■

To hear more from Vicki, including her favorite things about Chicago, her role as director of diversity for Ironworkers International, and the challenges of getting younger people interested in construction careers, visit modernsteel.com/podcasts.

business
issues

EFFECTIVE NETWORKING FOR INTROVERTED ENGINEERS

BY JENNIFER ANDERSON



Jennifer Anderson
(jen@careercoachjen.com) is a career strategist for the tech and engineering sector.

Here are four techniques to successfully navigate networking.

FOR SOME PEOPLE, networking can feel awkward, seemingly self-serving, discouraging, and even embarrassing.

Many people consider the term “networking” a dirty word and don’t like the potential “icky” feelings that come from attending a networking event. I agree that these events can feel awkward, but if you aren’t networking, then it’s going to be much harder for you to grow and develop as a professional. I promise you that networking has the power to help you in significant ways in both your professional and personal lives.

Here are four of my favorite techniques to more effectively navigate the world of networking:

Know the difference between the different types of networking. Networking is not isolated to attending a lunch-and-learn to hear a guest speaker share thoughts about an engineering-related topic. Networking can look like many different things, such as volunteering for a cause that you’re interested in, participating in an industry conference, taking additional college courses and getting to know your classmates through group projects, joining a running club, etc. Networking should never be just a bunch of people in a room trying to get other people’s business cards. Those types of events are definitely awkward!

I think it can be valuable to attend events where you learn from other professionals, but remember to take some time to evaluate how you want to make a difference in the world. For example, I have a colleague who is interested in STEM (science, technology, engineering, and math) initiatives to help adolescents learn about different ways they can engage with the world of science. As those kids come to realize how important STEM is, their eyes are open to more possibilities, and they find themselves more interested in math and science courses. The ripple effect is quite substantial in their education and career choices. In addition to the personal gratification, along the way my colleague has met numerous individuals that have helped her stay connected in her field in interesting ways that did not stem (pardon the pun) from a “typical” networking event.

As you find groups of people that are interested in similar hobbies, community causes, professional development, and so on, you will come to meet some interesting, passionate, and thoughtful people. That is when networking becomes fun and worthwhile.

Make time for networking. This might seem like a very basic technique, but if it’s so easy to do, then why don’t more people make time for networking? Typically, people tell me that they are “too busy” with work to network. Later, when they are ready to make a job change or ready to hire a new person to the firm, they find themselves with few strong connections—and at that moment, they wish they had made time for networking.



It's imperative to invest in the people within your professional circles so that you have time to get to know them, help them get to know you, and build mutual trust and respect. In the future, you'll be able to comfortably contact them when you're looking to make a job change or add to your team. If you find yourself feeling awkward at networking events, it's probably because you aren't participating in something that is interesting and thought-provoking to you.

Follow up on social media. After meeting new people, I recommend that you follow up and connect through social media. LinkedIn is my favorite place for connecting with other professionals. Sure, Facebook and Instagram are loads of fun, but LinkedIn is where most people think to connect professionally, so that's a better use of your time.

However, just because you're connected on LinkedIn or any other social media platform doesn't mean you're instantly best friends. You'll still need to nurture and cultivate relationships. Look for ways to stay in touch with people. Find articles to share that you think will be interesting to specific people, then copy the URL and share it in a message. Periodically check in with them; even a quick "hello" message goes a long way to keeping in touch.

One major benefit of being active on social media is that you will be exposed to other people who are likely outside your immediate circles. Join groups on LinkedIn to interact with other interesting people who live and work in different industries. Gaining a different perspective from other people is a great way to grow and develop—and in turn, you may offer a fresh perspective to someone else.

"So, what do you do?" Be ready to answer that all-too-common question because everyone asks it in one form or another. In answering that question, you have an opportunity to differentiate yourself from other people.

I will admit that I actually cringe when I hear people ask that question because it can be a conversation killer. After answering the question, people typically respond with, "And what do you do?" And then where do you go with the conversation? It's a dead-end question. Instead, consider asking other thoughtful questions that will help to create better, more in-depth conversations, such as:

"What brings you to this conference?"

"What's the most interesting thing you've learned at this trade show so far?"

"What does your firm specialize in?"

Conversations get much more interesting and far less awkward when you're discussing meaningful information beyond simply telling each other your job titles.

In the end, networking can lead to some great conversations and professional opportunities. Don't hold yourself back. Plan to attend. Go. Bring your business cards. Have a goal of meeting two to three people at the event (and don't just ask them what they do). Then wash, rinse, and repeat. Practice makes perfect. You'll find that over time, networking is truly a wonderful way to exchange ideas and meet and keep in touch with other professionals. This will, in turn, provide you the opportunity to build your professional network, enhance the probability for future opportunities in your career, and potentially add interesting, qualified people to your firm. ■

This article is a preview of the 2021 NASCC: The Steel Conference session "Even Introverted Engineers Can Network!" The conference takes place online April 12–16. For more information and to register, visit aisc.org/nascc.



Super Fast, Super Tall

BY STEVEN R.
WITKOWSKI, PE,
AND DARREN R.
HARTMAN, PE

HOW DO YOU CONSTRUCT one of the country's tallest buildings in a dense, bustling area next to one of the world's busiest transit hubs, and on an accelerated schedule? For one thing, start the steel connection design and modeling as early as possible.

One Vanderbilt, located at 42nd Street and Vanderbilt Avenue in Midtown Manhattan—practically at the doorstep of Grand Central Terminal—is the newest, and now second-tallest, office tower in New York City. Soaring to 1,401 ft, it includes 1.7 million sq. ft of space in 77 stories and was completed ahead of schedule and under budget.

Designed by architect Kohn Pedersen Fox Associates, the tower comprises four interlocking and tapering planes that spiral toward the sky. The structure, designed by engineer of record

Severud Associates, features structural steel floor framing, sloping columns, transfer structures, and outriggers around a concrete core—with the steel framing being erected first using a temporary steel core that was eventually embedded in the concrete core. This allowed the project to be erected as if there were no concrete core, thus maximizing the impact of steel's quicker erection time on the overall schedule.

With a building occupancy date planned for late 2020, erecting the superstructure, which comprises more than 25,000 tons of steel, began in the summer of 2017. As the building went through several design iterations in the fall of 2015/winter of 2016, Thornton Tomasetti's Construction Engineering practice was brought



Looking south toward Downtown Manhattan from one of the upper floors of One Vanderbilt, which encompasses 1.7 million sq. ft of space in 77 stories.

Max Touhey, courtesy of SL Green

onto the project to reclaim time in the steel schedule and ensure that the project's timely completion would not be jeopardized. The firm implemented its Advanced Project Delivery™ (APD) approach, which, along with the company's structural engineering expertise, provides early steel connection design and Tekla modeling services for mill orders and steel detailing.

Throughout the project, Thornton Tomasetti worked collaboratively with the design and construction team, starting 15 months before steel was to arrive on-site, bridging the gap between design and construction. During this initial phase, the company developed complex connection concepts for review and approval by Severud, identified and resolved any missing geometry information and framing conflicts on the early available documents as the Tekla model was being built, and gathered final force information required for connection design. An internal request-for-information process was used

In a city known for the tallest buildings and fastest pace of life in the country, a supertall skyscraper opens early thanks to an innovative steel connection design and modeling process.



Steven Witkowski is a principal and **Darren R. Hartman** is a senior principal and Construction Engineering practice leader, both with Thornton Tomasetti.

The tower comprises four interlocking and tapering planes that spiral toward the sky.

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between Thornton Tomasetti and Severud to track information between the two firms, with the former frequently offering design change suggestions or relief from prescriptive force requirements in specific instances that would help improve the steel connection engineering, detailing, fabrication, and erection.

The early connection and geometry information developed was issued to the structural steel bidders in summer 2016. To facilitate a greater understanding of the project requirements, a progress Tekla model and connection detail sketches illustrated some of the complexities and helped to solicit any of the bidders' recommendations for fabrication and erection enhancements. Additional details and progress Tekla models were issued throughout the bid process as the modeling advanced at the building podium.

Early connection engineering identified the need for solid steel forgings to accommodate transfer forces in two directions. These elements ranged in weight from three tons to 22 tons, with the largest being approximately 3 ft by 3 ft by 10 ft. These special conditions were detailed at three nodes in the lower part of the building and included in the bid documents to provide the bidders an early and greater understanding of what was required for the project, leading to more accurate and responsible bids. The structural steel fabrication contract was awarded to Banker Steel in October 2016.

The building was broken up into seven sequences for detailing. Each sequence had a scheduled advanced bill of materials (ABM) date and released for detailing (RFD) date. Sequence 1 is the lower part of the podium, from the foundation to Level 6. This sequence contains several 24-ft-deep transfer trusses, typically constructed of built-up box members, and 9-ft-deep plate girders, as well as horizontal bracing on several floors and the structural support of a sloping, hung ceiling over the lobby. Transfer truss connections were typically 26-in.-wide built-up nodes fabricated by laminating plates up to 6 in. thick. Sequence 2, the upper part of the podium, runs from Level 6 to Level 13 and includes a second set of transfer structures. Sequences 3 to 6 comprise 45 floors that are typical office framing floors with three outrigger levels. Finally, Sequence 7 highlights the glass-



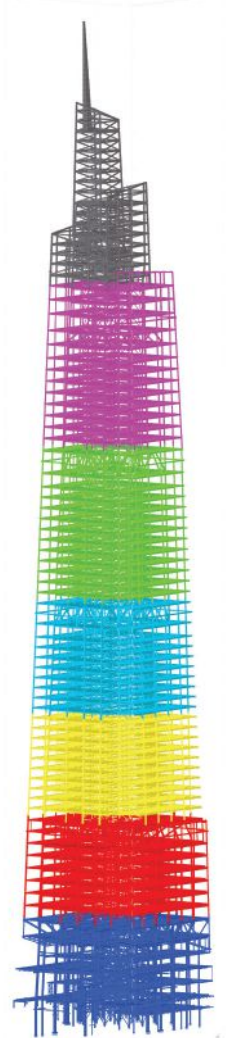
Max Touhey, courtesy of SL Green

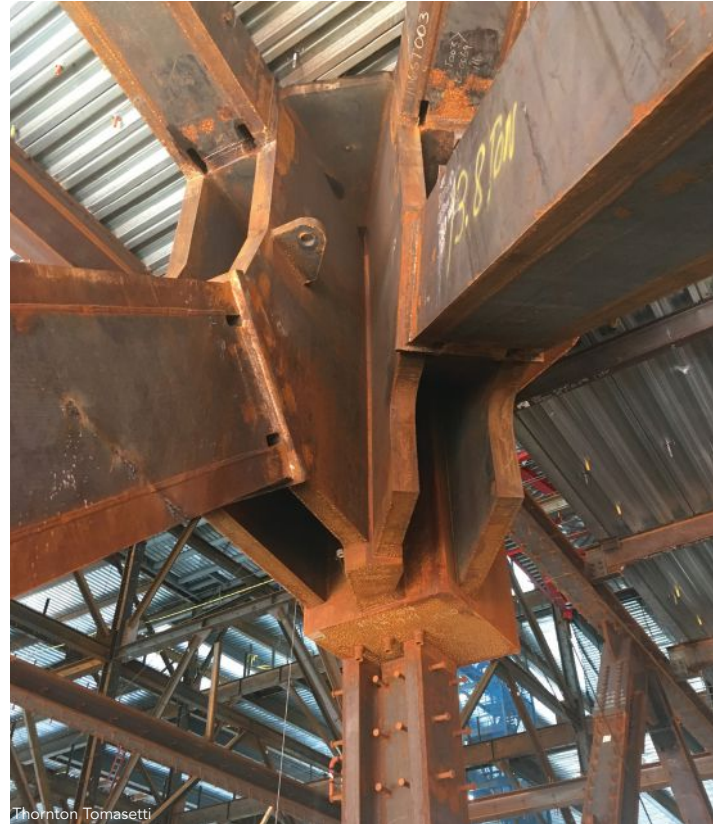
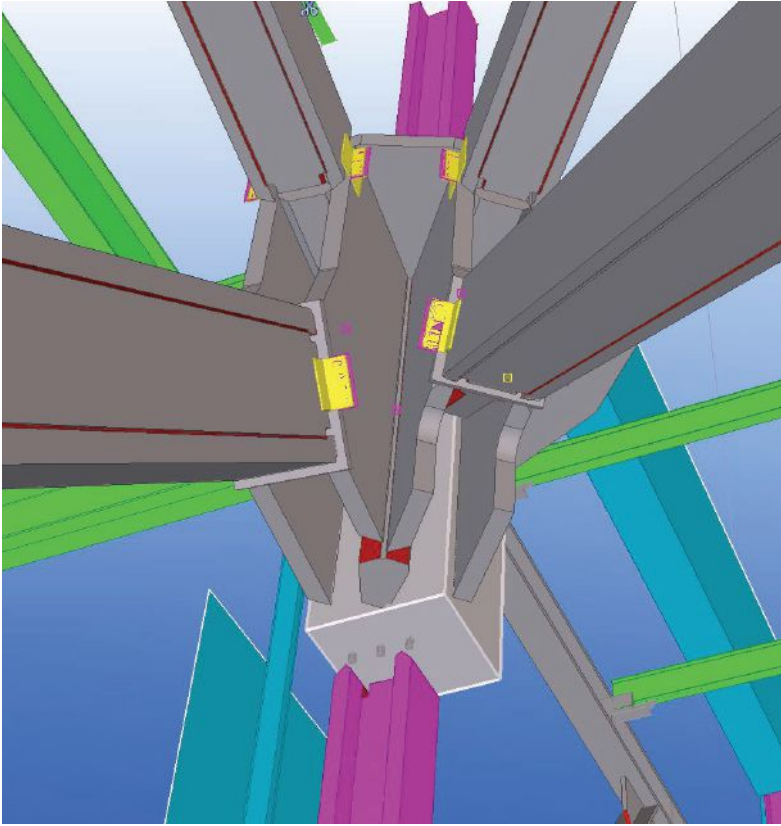
below: The building rises 1,401 ft and is now the second-tallest office tower (and fourth-tallest building overall) in New York.

right: The steel framing package was broken up into seven sequences for detailing.



Max Touhey, courtesy of SL Green

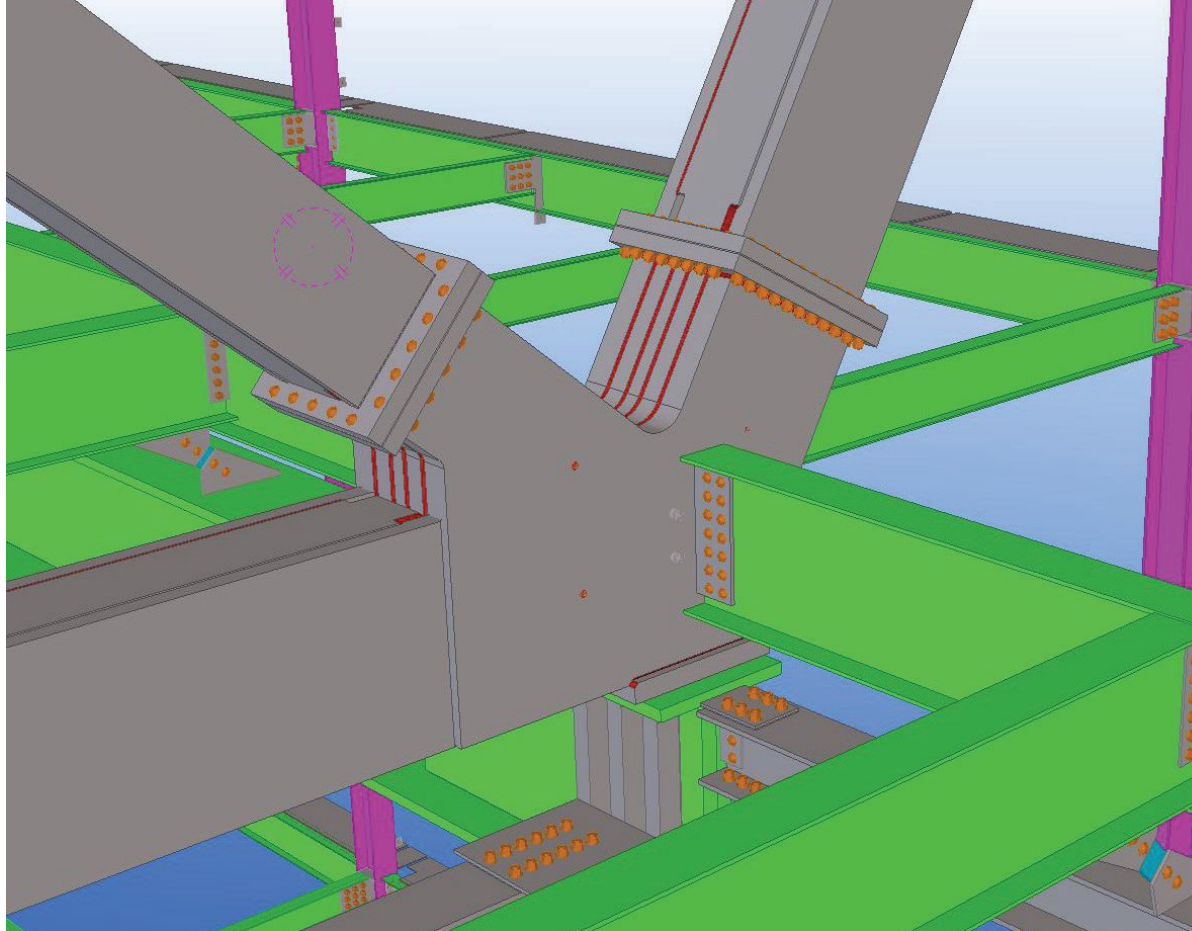
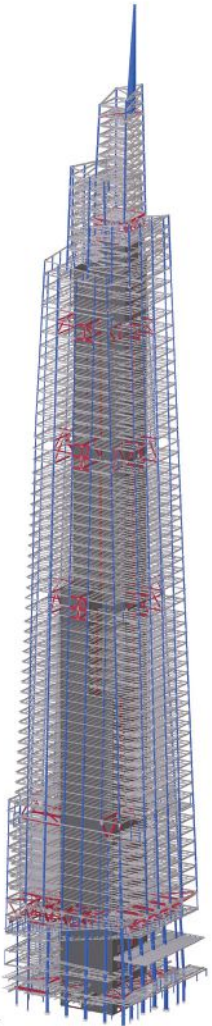




left: Steel framing for the entire tower totals 25,000 tons.

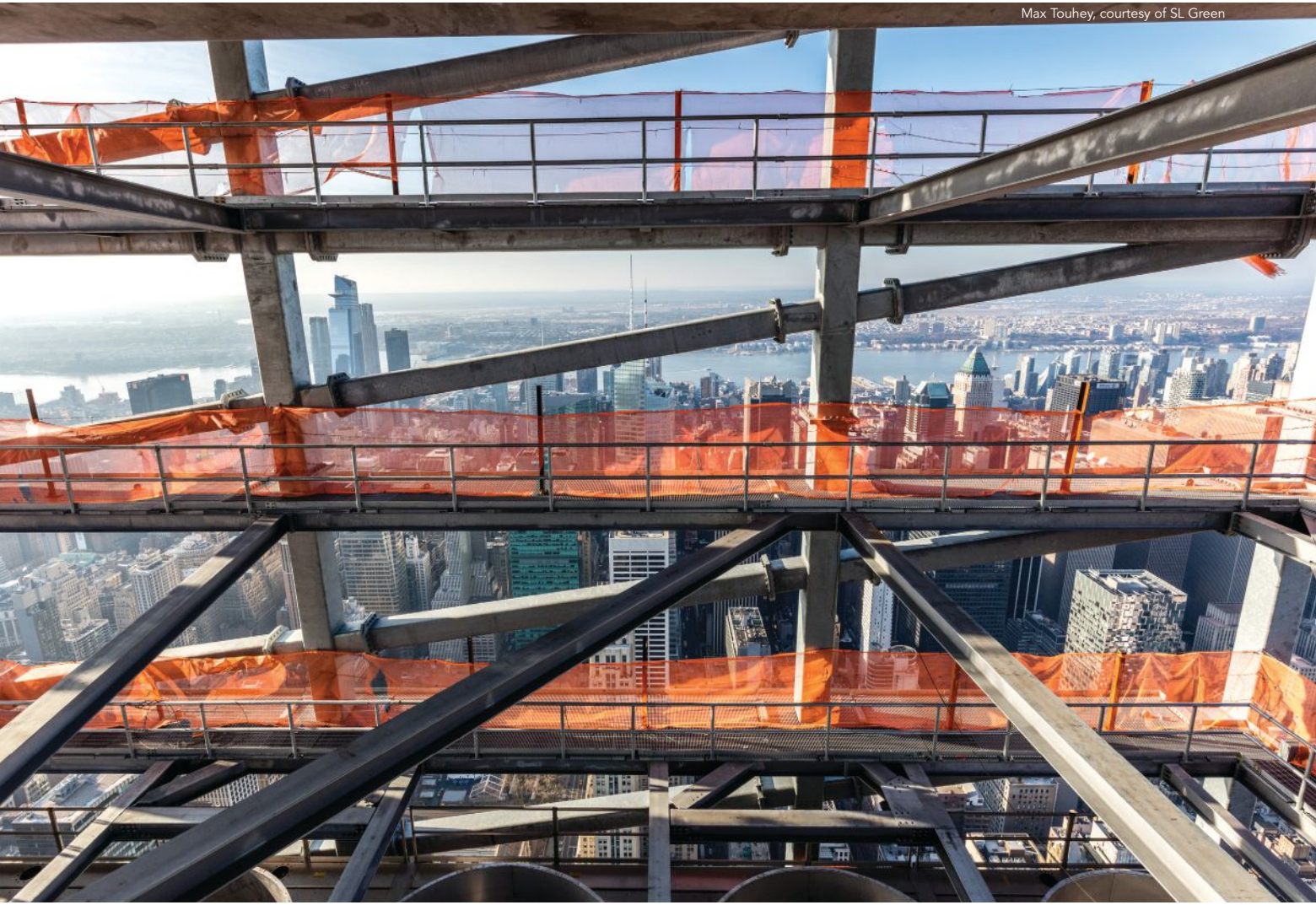
above: Solid steel forgings accommodate transfer forces in two directions and range in weight from three tons to 22 tons, with the largest being approximately 3 ft by 3 ft by 10 ft.

below: A model of a built-up 26-in.-wide transfer truss node.



Thornton Tomasetti implemented its Advanced Project Delivery service, which provides early steel connection design and Tekla modeling for steel mill orders and detailing.

Max Touhey, courtesy of SL Green



enclosed top-of-house structure with exposed hollow structural section (HSS) perimeter columns, horizontals, and diagonals up to HSS22x22 in size.

When the structural steel fabrication contract was awarded, an ABM model totaling approximately 11,500 tons (45% of the total project tonnage) for Sequences 1 and 2 was provided to fabricator Banker Steel, along with anchor rods and steel embedment plates for detailing. Three weeks later, an RFD model for Sequence 1, totaling 7,000 tons, was provided for shop drawing preparation. This was clearly one of the most complex sequences for the project and got the steel detailing and fabrication processes moving early on.

As connection engineering and modeling were developed, any outstanding information required between Thornton Tomasetti and the design team, as well as the connection design and Tekla modeling status, was reported to the entire team during weekly project meetings. The sequenced model delivery schedule that was included in the contract with Banker enabled the project team, both design and ownership, to provide answers and make key decisions in a timely manner. Staggering the model release dates and setting the dates early on supported the overall project schedule. Not only did this approach help streamline the flow of information, but it also allowed for early detection of potential conflicts and helped ensure that the project schedule could be accelerated.

With Sequence 2 released for RFD a month-and-a-half after Sequence 1, several steel pieces and a large amount of tonnage were already being turned into shop drawings for review and approval. Future sequences up the building were issued for RFD approximately every two months thereafter, and the RFD models allowed for the steel to be ordered to actual lengths rather than typical ABM order lengths where connections are not yet known, thus eliminating wasteful drop material. Those members with long lead times were ordered earlier based on the length information available at the time. Finalizing the connection design and modeling before mill order allowed for member sizes to be changed beforehand when beneficial to simplify connections or eliminate the need for costly reinforcing, such as increasing HSS walls that were locally undersized at connections for out-of-plane forces.

With the steel framing erected before the concrete core, Thornton Tomasetti also coordinated the concrete core rebar with any major steel connections. This included providing rebar couplers welded in the shop and/or holes in connection material for rebar to pass through in the field. This coordination helped reduce field issues between the steel and concrete trades, providing greater flexibility.



Thornton Tomasetti

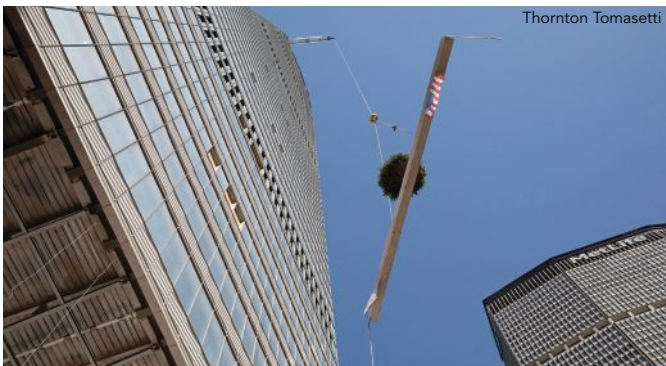
above: A steel truss node at street level prior to erection.

right: Transfer trusses in Sequence 2, constructed of built-up box members.

below: The steel framing for the tower topped out in September 2019.



Bess Adler, Thornton Tomasetti



Thornton Tomasetti

The structural steel shop drawings were reviewed by Thornton Tomasetti and the design team to confirm that they were prepared according to the details in the model, as well as designed for the proper connection forces. Any comments that could potentially affect the connection design were reviewed and coordinated prior to releasing the shop drawings back to the construction team. The shop drawing submittal process went smoothly, especially for a project of this size and complexity, with most drawings being approved or approved as noted on the first submission. By the time steel erection started in summer 2017, shop drawings had been submitted up to Level 31 and approved up to Level 27.

By preparing the Tekla connected model as the design progressed, the steel fabrication and erection schedule was reduced by at least eight months. Connection engineering and modeling were essentially completed in November 2017; all steel shop drawings were substantially submitted by late spring 2018; the framing topped out, with the last piece of the spire erected, in mid-September 2019; and the ribbon-cutting ceremony took place a year later.

They say the early bird gets the worm, and in the case of One Vanderbilt, getting an early start on connection design and modeling resulted in Manhattan's latest marquee super-tall building opening ahead of schedule, even in the midst of a global pandemic. ■

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Core Value

BY DEVIN HUBER, PE, PHD,
AND AMIT VARMA, PHD

The first in a multi-part series on SpeedCore covers what influenced the system, how it was developed, relevant research, design and fabrication considerations, and the reason for its name.

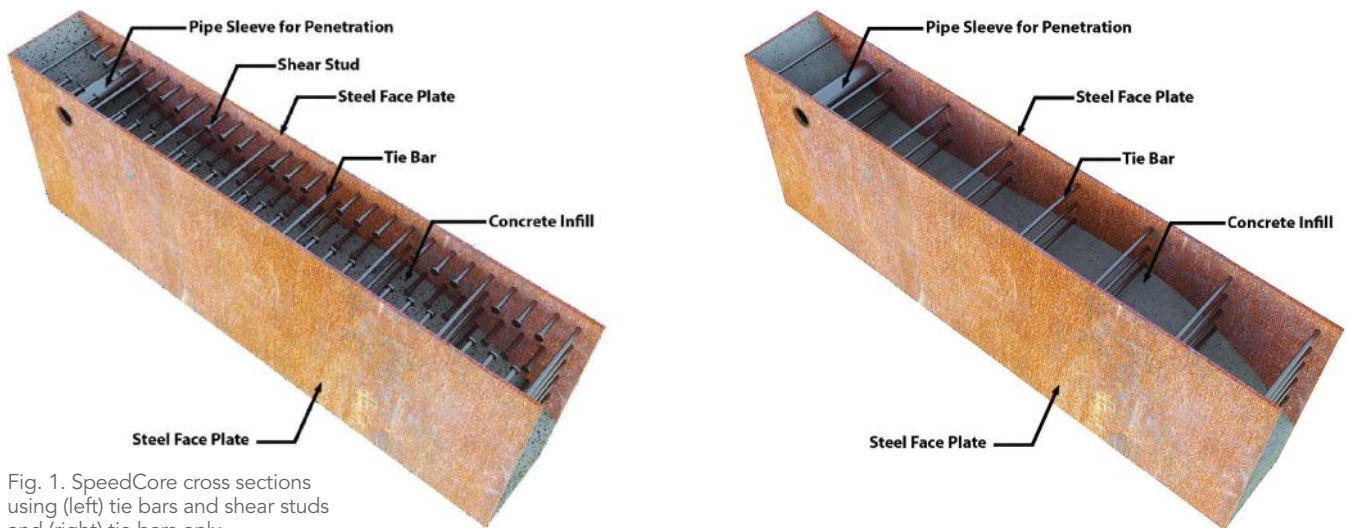


Fig. 1. SpeedCore cross sections using (left) tie bars and shear studs and (right) tie bars only.



Devin Huber (huber@aisc.org) is AISC's director of research.

Amit Varma (ahvarma@purdue.edu) is the Karl H. Kettelhut Professor of Civil Engineering at Purdue University's Lyle School of Civil Engineering and heads up all of Purdue's SpeedCore-related initiatives and research.

BY NOW, YOU MAY HAVE HEARD OF SPEEDCORE.

And if you haven't yet, you will soon—especially if you keep reading.

The system, whose longer, technical name is “composite steel plate shear walls/concrete filled (C-PSW/CF),” is intended for use as a lateral force-resisting system (LFRS) in multi-story steel construction, with Rainier Square Tower in Seattle being the first commercial steel building to use the system (see “Core Solution” in the February 2018 issue, available at www.modernsteel.com). SpeedCore is what is known as a “sandwich” structural system, which generally refers to a system composed of stiff outer layers with a softer layer in between. It can be used to replace reinforced concrete (RC) shear walls in building structures—and similar to RC shear walls, its walls may be coupled or uncoupled as required for the LFRS.

Here's how it works: Outer steel plates act as permanent formwork and reinforcing steel to an inner concrete core. The plates are tied together using round tie bars that can either be welded to the steel plate (as is the case at Rainier Square) or secured with a double nut configuration (see Figure 1 for the schematics of a typical SpeedCore panel). The system is optimized such that panels are fabricated in the shop and shipped to the job site, where they are connected into their final “core” configuration (at left in Figure 2). While sandwich construction is not a new concept, SpeedCore takes this methodology, literally, to new heights—to the tune of 850 ft in the case of Rainier Square (at right in Figure 2).

Sandwich-type construction has a fairly extensive history, seeing use in metal structures ranging from bridges to ships to nuclear power plants. Here, we'll provide a brief overview and background to sandwich construction, summarize background research that has helped develop the SpeedCore system, and describe some of the considerations for and benefits of using SpeedCore in a building structure.



Fig. 2. The Rainier Square project in Seattle, with (left) SpeedCore panels being erected and (right) framing approaching the building's full 850-ft height.



A Brief History of Composite Plate Shear Walls

Historically, there have been various iterations of sandwich systems. In “modern” history, the first large-scale application of sandwich systems in a civil engineering project was the construction of two railway bridges in Great Britain in the mid-1800s. Both bridges, the Britannia and Conwy Suspension bridges, were known as tubular bridges and were made of wrought iron built-up sections and configured with the rail traffic traveling within the structure. (A section and a rendering of the original bridge are shown in Figure 3.) These structures showed how a structure using stiff outer layers with a “softer” inner layer maximizes the efficiency of structural metals for carrying heavy loads.

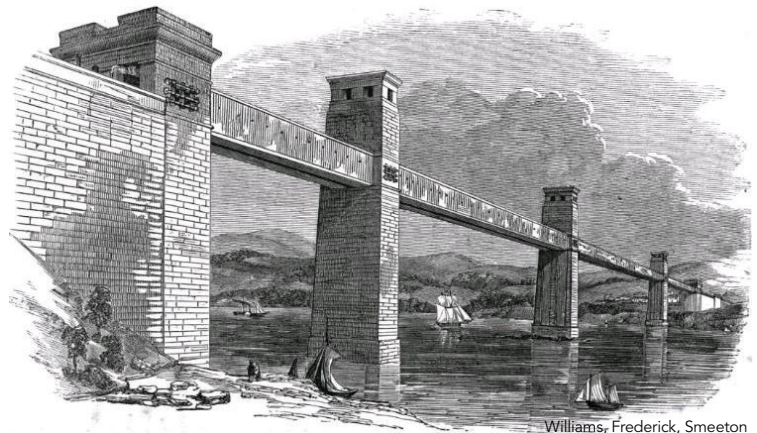


Fig. 3. A section of the original wrought iron tubular Britannia Bridge (left) standing in front of the modern bridge and (right) a rendering of the bridge circa 1852.



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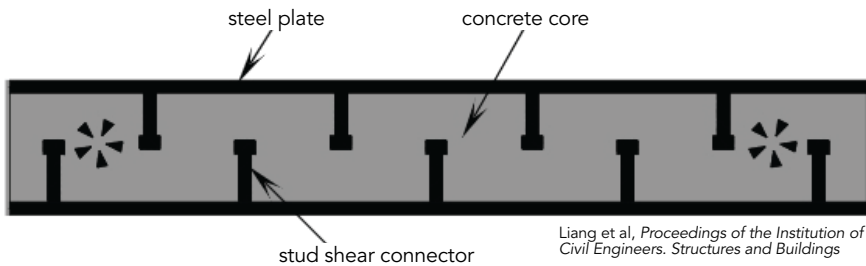


Fig. 4. A cross section of double-skin composite panel.



Fig. 5. A Friction-stir welding tool (left) to fabricate a bi-steel panel and a completed fabricated panel (right).



Fig. 6. A Corefast panel arriving at a site as a prefabricated unit (left) and being erected into a final configuration (right).

Over the years, numerous types of structural sandwich systems of various materials were developed for use in applications like ship hulls, airplane wings, and even some automotive uses. However, the first predecessors of the SpeedCore system were originally developed in Great Britain in the late 1980s and early 1990s for use in tunnel construction. These systems were known as double-skin composites and consisted of outer steel plates with welded shear studs and a concrete core between them (one of the early concepts shown in Figure 4).

A major breakthrough on double-skin composites came in the early 2000s when the British company Corus devised an efficient way to shop fabricate panels using a patented process where tie-bars were connected to the plates using a friction stir welding process. The resultant product from this process was known as a “Bi-Steel” panel and had the tie bars welded to the inside of the plates, creating a smooth outside surface (see Figure 5). These panel types were initially intended for defense applications as blast-resistant walls and eventually became more commonly used as core walls in buildings.

As the application of Bi-Steel panels became more extensive in building applications in Britain as shear walls, Corus rebranded this application as the “Corefast” system. The system gained some popularity in Britain in the early to mid-2000s, where it was used in several steel buildings as the LFRS. Much like SpeedCore, the system was panelized, most of the fabrication occurred in the shop, and final assembly occurred at the job site (as shown in Figure 6). While Corefast closely resembles the SpeedCore system, there are some key differences: SpeedCore does not use Bi-Steel panels, Corefast was never intended for seismic regions (SpeedCore can be used in both seismic and non-seismic areas), and SpeedCore is not patented and can be used by any designer and contractor on their given project.

The use of composite plate shear walls, referred to as steel-plate composite (SC) walls in safety-related nuclear facilities, has been evaluated since the early 1990s. In particular, the AP1000(R) nuclear power plant uses SC walls for almost all the containment internal structures and even for the external shield building. The internal containment structures (steam generator compartments, pressurizer, etc.) are designed for extreme load combinations, including seismic, accident thermal, and pressure loading. The external shield building is designed to resist seismic loads and aircraft impact, which is required by regulatory agencies following the 9/11 terrorist attack.



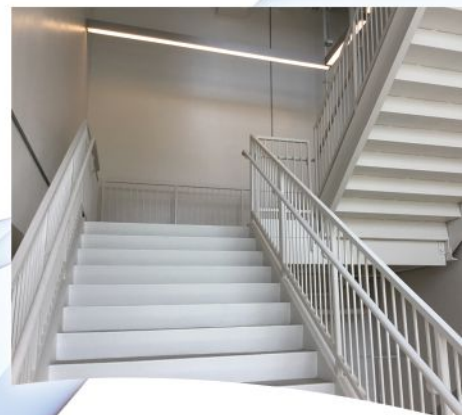
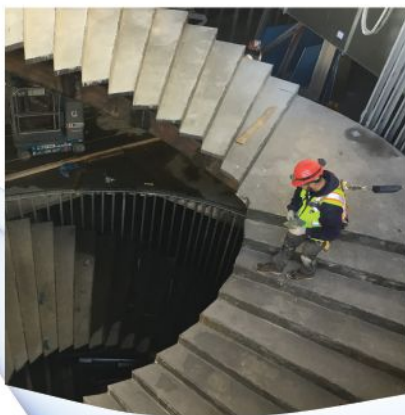
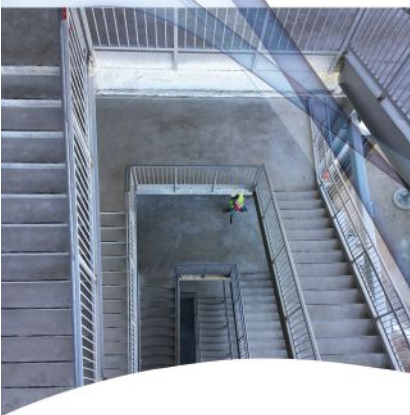
Fig. 7. Composite plate shear walls being used in the Vogtle Unit 3 Shield Building (a nuclear facility application).

SC walls have two steel plates located on the exterior and interior surfaces, resulting in armored construction, which is excellent for resisting missile/projectile impactive loading or impulsive/blast loading. The AP1000(R) nuclear power plant design, initially implemented in Sanmen and Haiyang, China, is currently

near completion of its first U.S. installation, in Vogtle, Ga. (see Figure 7). Several other nuclear power plant structures and other safety-related nuclear facilities are also using or considering SC walls due to their inherent advantages of construction speed, seismic performance, and resistance to impactive and impulsive load-



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ing. For several decades, there was no industry code or standard governing the design of SC walls in safety-related nuclear facilities. This became a major roadblock for their use, as well as for expedient review and approval by regulatory agencies. Project-specific design criteria, submissions, experimental results, etc., had to be developed by the vendor, and detailed review plans, audits, and expert panels had to be coordinated by the regulatory agencies. The designers had to rely on and build upon ACI 349 (*Code Requirements for Nuclear Safety-Related Concrete Structures and Commentary*) as the basis of design, but it was inapplicable and inappropriate for composite SC walls. Simply put, the technology needed an advocate.

AISC recognized this industry need and constituted an ad-hoc committee for the design of composite SC walls for safety-related nuclear facilities in 2006. Researchers from Purdue University and industry experts on the committee led a decade-long effort to generate and synthesize research results for SC walls, including extensive experimental results and numerical models/analyses. The results were instrumental in the structural design approval of nuclear power plants by regulatory agencies, which eventually led to their licensing and construction. The research was also used to generalize behavior and develop the first consensus code AISC N690 Supp. No. 1 (2015), which included extensive provisions for the design, modeling, analysis, and detailing of SC walls for safety-related nuclear facilities. In order to make the specifications more useful and user-friendly, the AISC Design Guide 32: *Modular Steel-Plate Composite Walls for Safety-Related Nuclear Facilities* (aisc.org/dg) was published to guide designers in effectively following the specifications.

SpeedCore Research

Initial research into the use of composite plate shear walls in commercial construction began at Purdue University as part of a Charles Pankow Foundation Project. The industry champion and primary motivator was Ron Klemencic, chairman and CEO of Magnusson Klemencic Associates. A 3:8 scale model of composite plate shear wall was constructed and tested at Purdue's Bowen Laboratory, and the experiments were used to investigate the stability of the empty steel modules, strain compatibility between steel and concrete, shear strength of horizontal splice connections, and behavior of the wall under cyclic loading. The results from the experimental and analytical investigations were used to develop the first design procedure for dual-plate composite shear walls (you can download a PDF of the results from Pankow at tinyurl.com/pankow-sc).

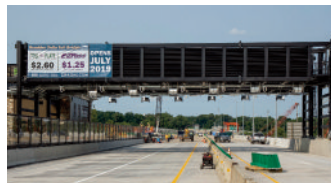
Further research, completed between 2014 and 2017, was performed to experimentally and numerically evaluate the in-plane shear behavior of composite plate shear walls. The results showed that these composite walls have significant in-plane shear strength, which includes the contributions of the steel plates and the concrete infill. A subsequent experimental investigation was conducted at the University at Buffalo, led by SUNY Distinguished Professor Michel Bruneau, on the flexural behavior of composite plate shear walls with semicircular or circular concrete-filled tube (CFT) boundary elements. The results showed that these walls can develop their plastic moment capacity and have excellent ductility. The out-of-plane shear behavior of composite plate shear walls was experimentally evaluated further, with the results showing that the out-of-plane behavior is similar to RC walls and depends on the shear strength contribution of the concrete and the tie bars.

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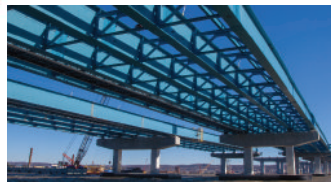
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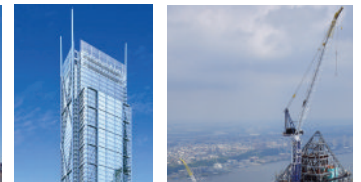
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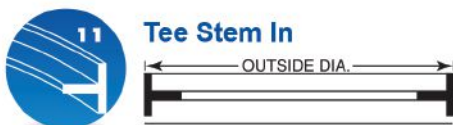
6 Beam The Easy Way (Y-Y Axis)
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36" x 925#

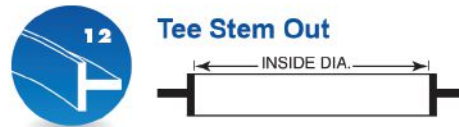
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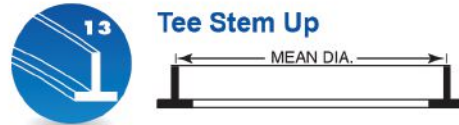
8 Channel Flanges In
 All Sizes


9 Channel Flanges Out
 All Sizes

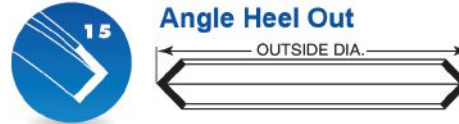
10 Channel The Hard Way (X-X Axis)
 All Sizes


11 Tee Stem In
 22" x 142¹/₂# Tee


12 Tee Stem Out We bend ALL sizes up to:
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13 Tee Stem Up
 22" x 142¹/₂# Tee

14 Angle Heel In
 8" x 8" x 1" Angle

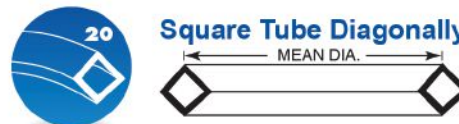
15 Angle Heel Out
 8" x 8" x 1" Angle

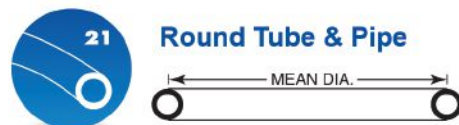
16 Angle Heel Up
 8" x 8"x1" Angle


17 Square Tube
 24" x 1¹/₂" Tube

18 Rectangular Tube The Easy Way (Y-Y Axis)
 20" x 12" x 5/8" Tube

19 Rectangular Tube The Hard Way (X-X Axis)
 20" x 12" x 5/8" Tube

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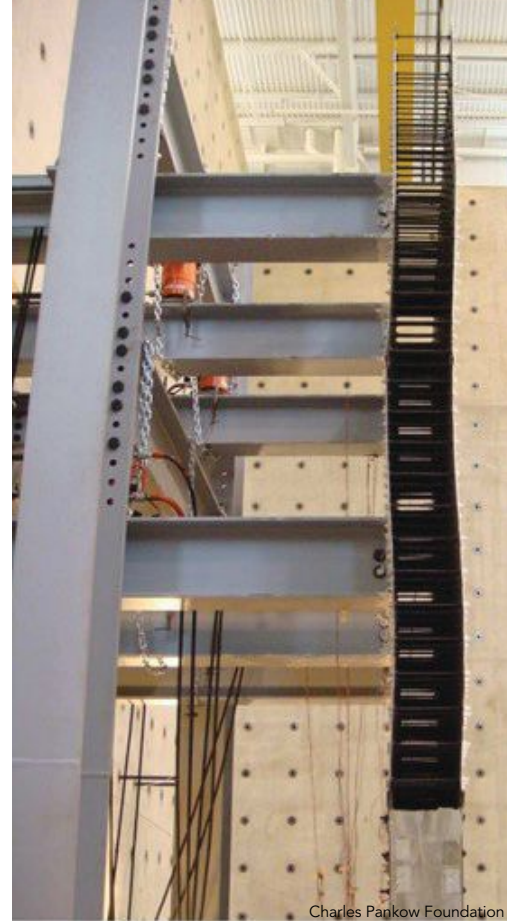
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Investigations were also performed on the stability of the empty steel modules before concrete casting between 2014 and 2019. The testing used experimental and analytical results to propose tie bar spacing requirements for composite plate shear walls. Additional investigation was performed on the local buckling of steel plates after concrete casting, as well as on the compressive strength of composite plate shear walls. They developed shear stud or tie spacing requirements (in the composite phase) to achieve non-slender steel plates that undergo yielding before local buckling. (Figure 8 shows a test specimen that was used to help develop tie bar spacing requirements.)

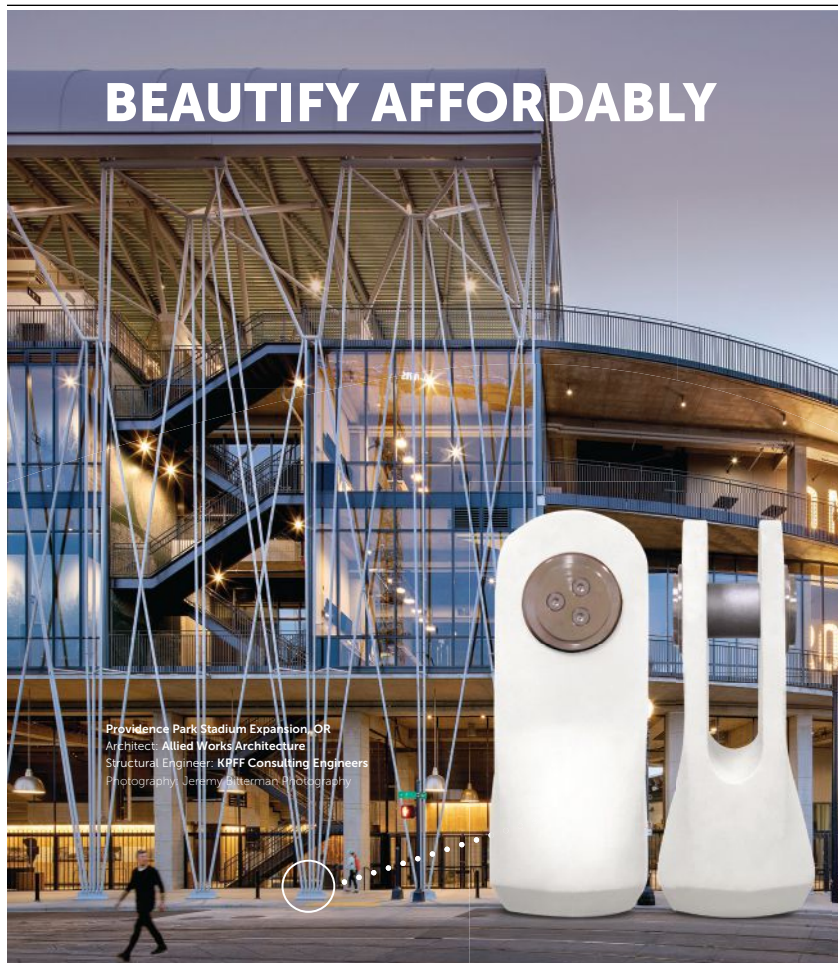
The missile impact behavior of composite plate shear walls was investigated using experimental results and numerical models to develop design methods for composite plate shear walls subjected to impactive loading. Also evaluated was the behavior of composite plate shear walls subjected to impulsive (blast) loading using experimental results and numerical simulations to develop design methods for walls subjected to impulsive loading. The work has helped demonstrate that composite plate shear walls perform well under extreme impulsive loads.

In addition to this research, Charles Pankow Foundation has partnered with AISC on multiple projects related to composite plate shear walls for use in commercial construction since 2016, all conducted at Purdue University and the University at Buffalo under the leadership of Amit Varma and Michel Bruneau. This research is far-reaching and includes work on seismic design, coupling beam connections, and fire engineering. With respect to fire engineering and designing composite plate shear walls, one of these endeavors included funding by the Steel Institute of New York with the work conducted at Purdue University. For this project, experimental and numerical research was conducted to evaluate the effects of standard fire loading on the thermal and structural behavior of composite plate shear walls. Different heating configurations, duration, and structural loading were considered. The results were used to develop standard fire-resistance rating (FRR) for prescriptive design and design equations for calculating the strength of walls subjected to elevated temperatures for performance-based design. Furthermore, the research showed a typical SpeedCore system using 18-in.-wide panels can readily



Charles Pankow Foundation

Fig. 8. A test specimen to calibrate tie bar spacing requirements for SpeedCore modules.



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Fig. 9. A SpeedCore specimen being tested under combined elevated temperature and axial loading at Purdue University.

achieve a two-hour fire rating without any additional fire protection applied (bare steel). This also assumes an axial load ratio of less than 17% and a slenderness ratio of the wall less than 20. (A photo of one of the test specimens from this research is shown in Figure 9.) Note that the online version of this article will provide additional information on the other related Pankow-AISC projects.

Standardization

The research results have been used to develop design provisions for SpeedCore. For example, design specifications for composite-plate shear walls in non-seismic applications are included in the forthcoming 2022 *Specification for Structural Steel Buildings* (ANSI/AISC 360; the current version is available at [aisc.org/specifications](https://www.aisc.org/specifications)). The public review ballot issued in 2020 included structural design specifications in Chapter I and fire-resistant design provisions in Appendix 4. In addition, seismic design provisions for SpeedCore will be included in the 2022 *Seismic Provisions for Structural Steel Buildings* (ANSI/AISC 341), also forthcoming. The public review ballot issued in 2020 included seismic design provisions for uncoupled SpeedCore in Section H7 and for coupled SpeedCore in H8. These provisions are also published as part of the National Earthquake Hazards Reduction Program (NEHRP) *Recommended Seismic Provisions for New Buildings and Other Structures* and are also in line for publication as part of ASCE 7-21, which includes the seismic design coefficients (R-factor, etc.) for coupled SpeedCore.

With all this work being done to get SpeedCore written into standards, you may be wondering if you can actually use SpeedCore for your next project? The short answer is yes. Beyond the Rainier Square Project, a recent project is underway in San Jose, and others are in the planning phases for construction on the east coast of America. In fact, SpeedCore has been approved by the New York City Department of Buildings for use in all boroughs of the city. Using the system will only get easier as the standards being updated continue to be published.

Design, Fabrication, and Erection Considerations

The benefits of using SpeedCore in commercial construction applications center around its speed of installation, which is rooted in its modular construction techniques. The ultimate goals with the aforementioned ongoing SpeedCore research and development work are to 1) develop clear specification language and design tools for engineers to efficiently design the system and 2) conduct further research that results in optimized fabrication techniques and details, efficient and constructable panel-to-panel splice details, and wall-to-foundation details that are simple to erect.

When it comes to specification language, there is some limited existing language in the current AISC *Seismic Provisions* for composite plate shear walls, but there is no language in the current AISC *Specification*. The intent of the proposed specification and provisions language is to provide the engineer with a systematic process for designing composite plate shear walls in both seismic and non-seismic applications.

In addition to these updates, an AISC Design Guide on SpeedCore is expected to be published this year. The guide will include specific design examples for composite plate shear walls—both seismic and non-seismic cases—and will also show scenarios using uncoupled walls and those using coupling beams in their design. The guide will also use the proposed specification provisions currently being balloted within the AISC *Specification* and *Seismic Provisions*.

From a fabrication and erection standpoint, SpeedCore does involve a learning curve. However, early involvement with a project's steel fabricator and erector will allow for most encountered concerns to be alleviated early on in a project. Items that should be considered and agreed upon early in a SpeedCore project include:

1. Layouts and placements of tie-bars either by welding, double-nuts, or other means and methods
2. Locations and methodology of completing splice connections in the field (welding versus bolted)
3. Modularization of panels—i.e., what can be reasonably be shipped to the site and handled in the field for erection (preferred module geometries)

4. If using a coupled wall system, an agreement on coupling beam layouts, geometry, and field installation requirements
5. Alignment with the authority having jurisdiction (AHJ) on any fire protection requirements
6. Wall-base-to-foundation connection. The interface between the base of a SpeedCore wall system to the building foundation can be complex and should be considered early in the design process with fabricator involvement when possible

Once some of these key items can be investigated and agreed upon by all relevant parties on a given project, a significant amount of optimization and speed can be realized for the project using the system. The core can then be detailed much like any other steel structure and the panels laid out and modeled so the fabricator can plan their fabrication sequencing.

The Speed in SpeedCore

The most pronounced benefit to using SpeedCore is, of course, the potential construction schedule improvement it can bring when properly specified, fabricated, and erected. The Rainier Square project realized a 43% schedule improvement by going to the SpeedCore system compared to a traditional reinforced concrete core. This quantified to an overall multi-million dollar savings to the building owner.

To best gain schedule improvements using SpeedCore, as has been mentioned previously, early fabricator and erector involvement is critical. This involvement includes collaboration with the engineer of record (EOR) to align on key elements such as module arrangements, wall thicknesses, tie bar size and spacing, splice details, and erection aids that may be required.

When considering implementing the system on a given project, the potential schedule improvements should be communicated as the primary driver for using the system. This is important when comparing the system to a traditional cast-in-place reinforced concrete wall, which may come with a lower up-front cost but a much longer construction schedule. These increased schedule times with a traditional reinforced concrete wall system will impact other trades on-site, increase general conditions cost, and impact overall construction efficiency.

Future articles in this series will go into further detail on specific topics of interest related to SpeedCore, including seismic design, wind design, fire considerations, and fabrication and erection considerations. ■



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Answers provided by Ben TARRIER, president of The TARRIER Steel Company:

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The TARRIER Steel Company was founded in Columbus, Ohio, by Fred TARRIER in 1919 after his return from World War I. The early years are not well recorded, but we have documents showing that Fred designed and fabricated some of the first steel joists. Four generations later, we have been able to maintain our status as a leading fabricator of structural steel in the central Ohio area, and we are still located on the same property.

Fred TARRIER, the company's founder.





above: TARRIER fabricated the steel for Marietta High School in Marietta, Ohio, seen here in 1965.

opposite page: The original TARRIER shop in 1921.

right: Ford Motor Company – Columbus Assembly Plant, also fabricated by TARRIER.



Your company has been able to weather challenges for over a century. How has this helped you weather the current pandemic?

Construction companies have to be flexible regardless of the situation. Our margins are so small and the risks so great that if you can't figure out a way to bend, then you will absolutely break. We like to react quickly but not so fast that we can't apply critical thinking and backward planning. The pandemic had a silver lining for us in that we were not set up for every office employee to work from home, and now we are. Instead of an employee having to use PTO because they have the sniffles, we have them work from home so they don't lose any time or come to work and get everyone else sick, and we don't lose any productivity. We strive to see how we can improve our processes when we're faced with a challenge.

What's the best advice you've received from past leadership at the company?

The biggest lesson that's been passed down is: Take care of our people and our people will take care of us. This applies to employees, customers, and vendors. We make every effort to do what's right, and we want companies and people we work with to do the same thing. There's no substitute for doing the right thing.

AISC turns 100 this year. How long has your company been involved with AISC?

We have been an AISC member since 2002 and became certified in 2009, and since then our production quality and output have gained tremendous ground because we have a quality manual we can refer to instead of simply "doing it the old way." It's not just a certification in order to gain access to bid on projects, it's an entire business transformation that gives everyone in the company a better understanding of quality standards and how the work is supposed to be performed. When we have a project or part of a project that we need to sub out to another fabricator, the first thing we do is look at the AISC list of certified members, even if the project doesn't call for an AISC fabricator. We want to know that the fabricator can maintain the level of quality that we are expected to adhere to. ■



above: The National Veterans Memorial Museum in Columbus, which opened in 1918. TARRIER fabricated the project's 400 tons of steel.

left: The 500 Neil Avenue office building in Columbus, for which TARRIER completed steel fabrication in 2020.

conference preview

THINKING INSIDE THE (BIG) BOX

BY SHELLEY R. CLARK, SE, PE, AND
TRAVIS P. CORIGLIANO, SE, PE



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is an associate, both with
Magnusson Klemencic Associates.

Four insights for successfully repurposing anchor big box stores.

LET'S FACE IT: America's retail sector is experiencing a host of profound challenges—from the ongoing and increasing shift from in-person to online shopping to the simple fact that the country is “over-retailed.”

While the COVID-19 pandemic's economic impact is certainly another challenge, malls were already struggling prior to its onset; the pandemic just forced property owners to consider new development strategies sooner.

Hundreds of department stores—once anchor tenants at shopping malls across the country—have closed their doors forever. Typically, large anchor stores account for as much as 30% of a mall's square footage, and more than half of all mall-based department stores are estimated to close by the end of 2021. Across the country, this demise of the anchor store is having a large impact on the health of mall properties. A quarter of the 1,000-odd remaining U.S. malls now look likely to close in the next three to five years, according to Coresight Research.

Repurposing anchor stores that have gone dark offers a promising solution for the new retail landscape—but it's no simple task. While most anchor box stores may look the same from the street, the structural systems supporting those unadorned exteriors can vary drastically.

An understanding of the underlying structure is vital to informing the costs and design approaches for revitalizing these structures. Here are four key insights to consider when determining if a structure should be repurposed or torn down and rebuilt.

Assess the Existing Structural System

Answering the following questions can help property owners, developers, and design teams determine if a vacant big box store is a good candidate to be repurposed and redeveloped:

- Is the existing column grid spacing generally compatible with the planned use for redevelopment?
- Does the existing floor have the live-load capacity required for the planned future use? If not, can a simple strengthening method be applied to achieve the higher load required?
- Is the existing exterior wall a nonstructural façade element that can easily be removed and replaced (as opposed to a structural, load-bearing shear wall, which is part of the primary structure)?

In general, if two or more of these questions can be answered “yes,” the structure should at least be considered for repurposing and redevelopment.

Understand the Advantages of Steel-Framed Buildings

Anchor boxes with beams and columns made of wide-flange steel often provide the easiest and most economical redevelopment opportunities. These buildings typically include steel lateral systems of braced frames or moment frames, which allow the exterior skin to be removed and replaced with no impact to the primary structure.

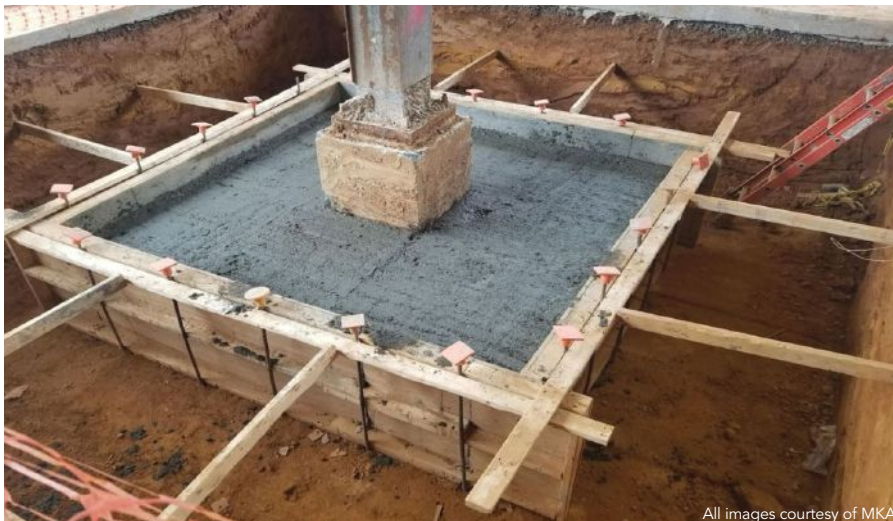
When wide-flange strengthening is required, increased capacity for individual members can often be provided by welding new steel elements to the existing framing. For projects looking to increase floor-loading capacity, consider the following:

- For composite wide-flange floors, welding steel plates or WT sections to the underside of the existing framing is often the most efficient way to provide increased bending strength.
- For non-composite sections, welding steel plates to the underside of the top and bottom flanges of the existing steel beams will typically provide the most efficient increase in bending strength.
- For column strengthening, attaching side plates to “box out” a column will provide a larger radius of gyration to allow for increased axial load or longer unbraced lengths.

When adding new floor openings in steel-framed buildings, it is important to evaluate whether there is sufficient ceiling space for an “underslung” beam. An underslung beam can be connected into existing framing (likely strengthened) before any existing structure is cut. With the underslung beam in place, the existing floor can be removed without any temporary shoring or impact to the floors below.

Recognize the Opportunity to Improve Concrete Buildings with Steel

It is common for anchor box stores built in the 1950s and 1960s to have exterior load-bearing walls supporting floors—whether mild flat slab, mild slab with beams, or hollow-core planks with concrete topping—made of concrete. While these buildings tend to have reserve capacities in their floors and walls, *understanding* those reserve capacities can be difficult without full and legible existing drawings that provide concrete design strengths along with reinforcing quantities and locations. Field tests can provide this information, but such tests can be time-consuming and costly.



All images courtesy of MKA



Three examples of steel-framed buildings that have been structurally retrofitted.

conference preview



A new underslung beam positioned beneath existing wide-flange steel framing.

Once enough information is collected to determine the baseline strength of the concrete building, several techniques can be used to redevelop and repurpose these box building types—and rarely does the list include adding new concrete to the concrete box.

Fiber-reinforced polymer (FRP) added in the tension regions of the slab—typically underside of the middle of the bays and over the top of support locations—can enhance the distributed floor-load capacity. Alternatively, if there is enough head clearance, steel beams can be added underneath the existing slab to reduce the slab span lengths.

If redevelopment conditions require new and concentrated loads, floor openings, or floor extensions, FRP is not likely to be helpful. Instead, steel beams can be added below the slab to directly support the new conditions. With some creative detailing, the new steel beams can frame directly into the existing vertical concrete structure, eliminating the need for new columns and foundations.

Understand the Challenges of Open-Web Steel-Joist Buildings

When looking to retrofit big box stores built between the late 1960s and early 1980s, open-web steel joist (OWJ) floors are almost certain to be encountered, especially in the Southeast. In general, OWJ floors are structurally efficient but lack reserve capacity. Redevelopment and retrofitting challenges inherent in OWJ floors include:

- The actual OWJ elements are manufacturer-provided items, and their final designs are not often kept with other structural base building drawings.
- There are no guarantees OWJ-framed buildings have steel lateral systems. Rather, these structures are often supported by load-bearing exterior concrete masonry unit (CMU) walls.
- It is difficult to efficiently strengthen, cut, and reconfigure OWJs for new floor openings. New floor openings likely require peeling back all slab and OWJ framing to their nearest unaffected beam or girder and reframing full, new bays with wide-flange steel.



above and below: A floor extension using new steel framing.



Speedier and Sustainable

While redeveloping and repurposing anchor big box stores can be complex and somewhat costly, many of these projects are generally faster to complete and more sustainable than tearing down an existing structure and building a new one. Early due diligence is required on the part of the structural engineer to help the project team decide up front whether the anchor box can be economically repurposed.

Will the future of mall properties be driven by designers thinking *outside* the box, tearing down existing structures and bringing whole new visions to life—or rather those who think *inside* the box, working to redevelop and repurpose existing big box stores that, while vacant today, can be reshaped with new purpose and potential tomorrow? ■

This article is a preview of the 2021 NASCC: The Steel Conference session “Rebuilding the Big Box.” The conference takes place online April 12-16. For more information and to register, visit aisc.org/nascc.



Using structural steel to enhance OWJ framing for added load.

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www.shortspansteelbridges.org

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www.sdi.org

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Steel Joist Institute

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ph: 843.407.4091
www.steeljoist.org

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Structural Stability Research Council

Chicago, Ill.
ph: 312.670.7015
www.ssrcweb.org

The Structural Stability Research Council is a technical organization that focuses on the state-of-the-art understanding of the impact of stability related issues on the analysis, design, and behavior of metal structures. SSRC is comprised of engineers, educators, and industry members with an interest in stability related issues.



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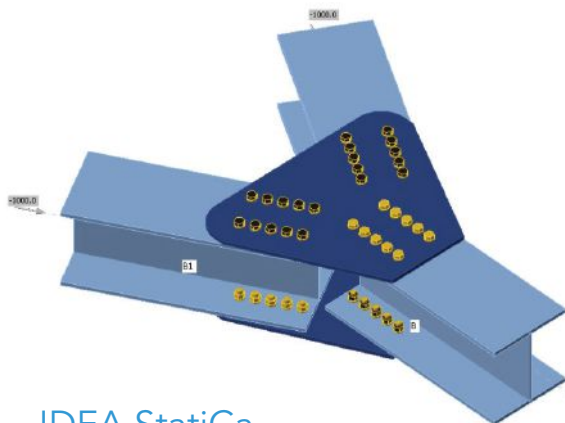
new products

This month's offerings include a moment connection solution with new sloped beam and multi-axis capabilities, a steel connection design software package that recently received its patent, and a downsized robotic welding gantry.

Simpson Strong-Tie Yield-Link

The Yield-Link moment connection for structural steel now features newly validated sloped beam and multi-axis moment applications. Designed to absorb forces in a seismic or high-wind event, the moment connection requires no field welding and allows beams to be designed without supplemental lateral bracing. At the roof level, structural steel beams are often sloped to accommodate drainage, and two new solutions provide greater design flexibility and streamlined field connections when using the Yield-Link in slope beam applications. Designed for corner, three-sided, and four-sided column connections, Yield-Link multi-axis moment connection solutions provide a connection that resists moment demand in both directions of the column.

For more information, visit www.strongtie.com.



IDEA StatiCa

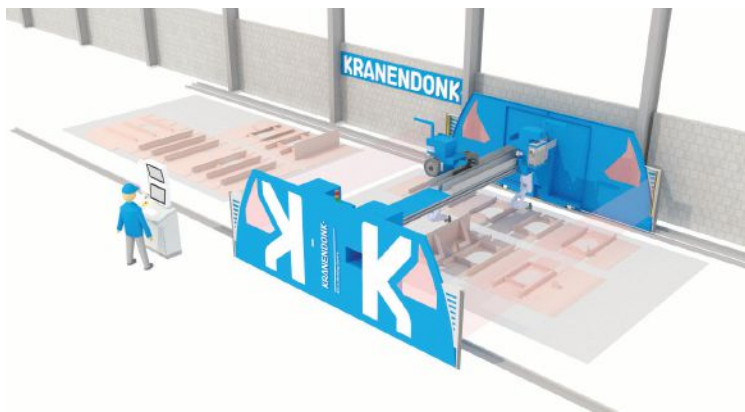
IDEA StatiCa is a steel connection design software package for all types of welded and bolted connections, base plates, footings, and anchoring. It also enables designers to solve buckling and stability of steel members. And it just became patented! The software can now design and code-check all types of connections, not just those in design guides, thanks to its component-based finite element method approach, which effectively combines all the code equations and conditions with finite elements, breaking down the topology and loading limits of older methods.

For more information, visit www.ideastatica.com.

Kranendonk Artemis Micro Welding Gantry

The Artemis Micro Welding Gantry—which can fit in a 40-ft container—was developed for maximum efficiency in smaller spaces. The gantry is constructed so that the side covers can be positioned along the gantry beam. Even the robots do not need to be removed for transport. The solution can be implemented into an existing production process and become fully operational within days. The system comes with one or two robots, with the second robot increasing production capacity and enabling optional double fillet welding (with both robots welding on the same stiffener). This optimizes welding and product quality by limiting distortion from heat input.

For more information, visit www.kranendonk.com.



Welcome to Safety Matters, which highlights various safety-related issues. This month's topics are brain injuries and hard hat replacement.

Brain Injuries

Traumatic brain injuries (TBIs) result in millions of emergency room visits, hospitalizations, and deaths per year in the U.S.—2.87 million such incidents in 2014, as a matter of fact. A TBI is caused by a bump, blow, or jolt to the head or a penetrating head injury that disrupts the normal function of the brain. Not all blows or jolts to the head result in a TBI, and the severity of a TBI may range from “mild” (a brief change in mental status or consciousness) to “severe” (an extended period of unconsciousness or amnesia after the injury).

A recent CDC (Centers for Disease Control and Prevention) report, *Deaths from Fall-related Traumatic Brain Injury*, found that in most states, the rate of fall-related TBI deaths has increased significantly, and the country as a whole saw a 17% increase in fall-related TBI deaths from 2008 to 2017. The most notable increases in fall-related TBI deaths were observed among persons 75 years of age and older and persons living in the most rural counties. This report highlights the importance of expanding efforts to stem the continued growth in the rate of fall-related TBIs among at-risk groups.

Effects of TBIs can include impairments related to thinking or memory, movement, sensation (e.g., vision or hearing), or emotional functioning (e.g., personality changes, depression). Falls are a leading cause of TBIs and typically account for nearly half of all TBI-related emergency room visits, and being struck by or against an object is another leading cause of TBI-related emergency room visits. Concussions are a common TBI, and it's important to recognize the signs:

- One pupil larger than the other
- Drowsiness or inability to wake up
- A headache that gets worse and does not go away
- Slurred speech, weakness, numbness, or decreased coordination
- Repeated vomiting or nausea, convulsions or seizures (shaking or twitching)

- Unusual behavior, increased confusion, restlessness, or agitation
- Loss of consciousness (passed out/knocked out)

Even a brief loss of consciousness should be taken seriously, and anyone who has been thought to suffer a concussion should be seen by a healthcare provider.

Properly used fall-arrest equipment and hard hats are key to minimizing TBIs in steel construction work, whether in the shop, on the job site, or somewhere in between.

Hard Hats

When it comes to hard hats, some companies are switching to helmets instead. ANSI Z89.1 Type II helmets provide protection from lateral impact in addition to the top impact protection provided by Type I hard hats. Many also have chin straps, which help improve their record of preventing injury. Note that not all climber-style helmets are ANSI Z89.1 Type II. Look for the type marked on the helmet.

The current standard for head protection is ANSI Z89.1-14 (R2019). Each ANSI

Z89.1 hard hat must have the following information clearly marked inside the hat:

- Manufacturer's name
- ANSI standard that the hard hat conforms with, such as “ANSI Z89.1-2009”
- ANSI type (Type I or II) and class designation (G, E, or C)
- Size range for fitting
- Date of manufacture
- If the hard hat meets Z89.1-2009 or later, it must also contain the following:
 - Two arrows curving to form a circle when the helmet can be worn forwards or backward
 - LT if the helmet is designed to provide protection at low temperatures -22 °F (-30 °C)
 - HV if/when the helmet meets all requirements for high visibility

Hard hats must be replaced if they show signs of damage (dents, cracks, penetration, or fatigue due to rough treatment), and it is essential to inspect hard hats for damage and signs of fatigue each time they are used. In addition to visual inspections, another way to test a hard hat is to grasp it in two hands and apply force by squeezing the hat. If you hear creaking or other unusual sounds, it is time to replace the hard hat.

While OSHA has no specific provision for an expiration date, manufacturers are allowed to determine if their equipment expires on a specific calendar date. In lieu of an expiration date, a generally accepted rule is to replace the support strap yearly and to replace the hard hat every five years. Harsh chemicals and extreme temperatures can make a hard hat degrade more quickly. UV light exposure deteriorates shells and is a particular concern of those who work outside. Be sure to check with the manufacturer for guidelines on hard hat replacement and maintenance.

We are always on the lookout for ideas for safety-related articles and webinars that are of interest to AISC member companies. If you have safety-related questions or suggestions, we would love to hear them. Contact us at schlaflly@aisc.org. And visit AISC's Safety page at aisc.org/safety for various safety resources. In addition, AISC has established its own resource page with information on employment, contract, and safety issues regarding COVID-19. It's at aisc.org/covid19.



Type II hard hat and helmet.

news & events

BRIDGES

AASHTO Selects Steel Press-Brake-Formed Tub Girder as 2021 Focus Technology

The American Association of State Highway and Transportation Officials (AASHTO) Innovation Initiative (AII) selected the steel press-brake-formed tub girder (PBTG) bridge system as a 2021 Focus Technology. AII will invest time and resources to accelerate the system's adoption among AASHTO member associations, local agencies, and industry partners to improve U.S. infrastructure.

Researched and tested by the Short Span Steel Bridge Alliance (SSSBA)—of which the National Steel Bridge Alliance (NSBA) is a member organization—the PBTG is an accelerated bridge construction system consisting of modular galvanized shallow trapezoidal boxes fabricated from cold-bent structural steel plate. Anchor Bay Drive

along Lake St. Clair in Clay, Mich., a 2020 Prize Bridge Award winner, is an example of a PBTG bridge; see the July 2020 issue in the Archives section at www.modernsteel.com.

“We are honored to have PBTG selected and thank the AII selection committees for their recognition of its economical and sustainable benefits,” said David Stoddard, senior steel applications engineer for SSAB Americas and chairman of the SSSBA. “Since AII advances innovation from the grassroots up, this opportunity will fast-track this economical system further into the mainstream, providing an optimal steel solution to meet the nation's infrastructure challenges.”

Learn more about the PBTG system at www.buildusingsteel.org.

PUBLIC REVIEW

Several AISC Standards Available for Public Review

A draft of the 2022 edition of the AISC *Specification for Structural Steel Buildings* (AISC 360) is available for public review until March 5, 2021. This is the second public review period for this specification, which is expected to be completed and available in late 2022.

Also, the 2022 edition of the AISC *Seismic Provisions for Structural Steel Buildings* (AISC 341) is available for public review from March 5 until April 2.

In addition, two new AISC standards are available for public review until April 12: the AISC *Specification for Structural Stainless Steel*

Buildings (AISC 370) and the AISC *Code of Standard Practice for Structural Stainless Steel Buildings* (AISC 313). Both standards are expected to be available by the end of 2021.

All draft standards and related review forms can be downloaded from the AISC website at aisc.org/publicreview. Hard copies of review drafts are available (for a \$35 charge each) by calling 312.670.5411. Please submit comments using the forms provided online or to Cynthia J. Duncan, AISC's director of engineering (duncan@aisc.org), by the dates indicated for consideration.

corrections

The Individual Awards news item in the February 2021 issue listed the incorrect award type for a handful of winners. Please note the following corrections:

- Dennis Noernberg, Jeffrey Packer, and Rafael Sabelli received Lifetime Achievement Awards, not Special Achievement Awards.
- Scot Becker and Dale Ison received Special Achievement Awards, not Lifetime Achievement Awards.

In addition, the January 2021 article “Century of Service” (www.modernsteel.com) included a list of AISC member fabricators that have been in operation for over 100 years. **American Bridge Company** has been in existence since the 1800s, and **AFCO Steel**, now part of **W&W/AFCO Steel**, was founded in 1909. Both companies should be included in this list. If you happen to know of another 100-year-old fabricator that wasn't on the list, please email Carly Hurd at hurd@aisc.org.

People & Companies

Bridge engineering and supply company (and AISC member)

Acrow announced that it has provided 26 modular steel bridges to Administração Nacional de Estradas (ANE), the National Road Administration of Mozambique. The bridges will be installed in rural areas of Mozambique to restore vital transportation routes damaged by cyclones in 2019.

Canam Steel Corporation (CSC), an AISC associate member and manufacturer of open-web steel joists and steel deck products, announced that **Chris Ervin** has become regional general manager (RGM) of the Southeast Region. With nearly 30 years of professional experience in the industry, Ervin will be responsible for expanding CSC's new brand identity, spurring from its recent separation from its former parent company.

Thornton Tomasetti announced that **Brian G. Morgen, SE, PE, PhD**, has joined the firm as vice president and Seattle office director. Morgen has more than 16 years of experience in the Seattle market and an extensive background in structural analysis and design and project management. He has collaborated with project teams on many technically challenging structures, including office and residential towers, healthcare facilities, civic and cultural centers, and mixed-use complexes. Morgen is a specialist in performance-based seismic design and played a key role in the design of several high-performing buildings in earthquake-prone areas in the Western U.S. and Asia.



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Peddinghaus 643E Anglemaster, 6" x 6" x 1/2", 200 T Shear, 66 T Punch, Fagor CNC, 40' Conveyor, 1991, #30325

Franklin 4280, 8" x 8" x .75" Max. Capacity, 100 Ton Punch, 30' Infeed, 25' Outfeed, Auto Loading, 2010, #31230

Peddinghaus FPB-500/3C, 150 Ton Triple Gag Punch, 20.8" x 40' Plate, 200 Amp Plasma, Control & Drive Upgrade '09, #30803



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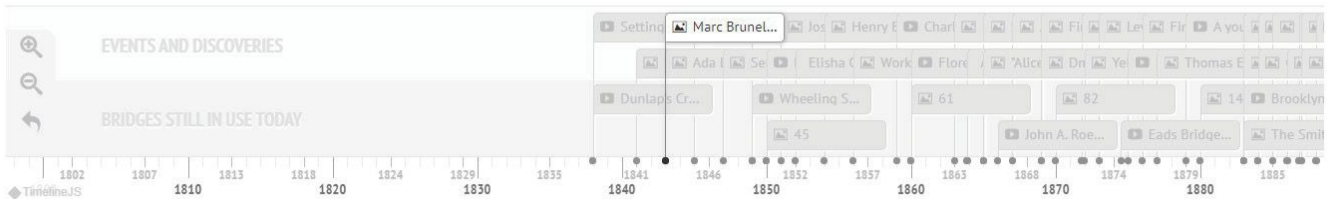
1843 MARC BRUNEL'S TUNNEL UNDER THE RIVER THAMES OPENS

Six steel bridges that are still in service today were open to traffic when it happened.

It is the world's first tunnel under a river and took 18 years to complete. The East London Railway Company purchased the tunnel in 1865; it became part of London's Underground system.

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TEST OF TIME

A LOT HAS HAPPENED since the first U.S. steel bridge still in service—Dunlap's Creek Bridge in Brownsville, Pa. (technically a cast iron bridge)—opened in 1838. And today, there are thousands of 100-year-old (and then some) steel bridges across the country.

An interactive timeline on AISC's website lets you take a stroll back in time—to 1838 and even earlier—highlighting major world events and aligning them with the number of U.S. steel bridges, still in service today, that were already open to traffic when they happened. For example, in 1843, Marc Brunel's tunnel opened under the Thames River in London. It was the world's first tunnel under a river and took 18 years to build, and eventually became part of the London Underground mass transit system. As impressive as that feat was, there were already six U.S. steel bridges open to traffic at the time that are still in operation today.

You can jump in the time machine at aisc.org/timeline.

And speaking of steel entities that have hit the century mark, AISC turns 100 this year! Check out the "Centennial Content" section at www.modernsteel.com for content related to this milestone. ■

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